JOINT HIGHWAY RESEARCH PROJECT
JHRP-87/12
Informational Report
PLACEMENT RATES FOR HIGHWAY EMBANKMENTS WITH VERTICAL AND HORIZONTAL DRAINAGE

A.G. Altschaeffl
S. Thevanayagam
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A.G. Altschaeffl
S. Thevanayagam
Informational Report on Placement Rates for Highway Embankments with Vertical and Horizontal Drainage

Prepared for State of Indiana, Department of Highways

By A. G. Altschaeffl S. Thevanayagam

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# TABLE OF CONTENTS

<table>
<thead>
<tr>
<th>Section</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>1. Introduction</td>
<td>2</td>
</tr>
<tr>
<td>1.1 Capabilities of the Program</td>
<td>2</td>
</tr>
<tr>
<td>2. Description of the Program</td>
<td>3</td>
</tr>
<tr>
<td>2.1 Main Program Modified Sand</td>
<td>4</td>
</tr>
<tr>
<td>2.2 Subroutine APPROX</td>
<td>18</td>
</tr>
<tr>
<td>2.3 Subroutine DETFS</td>
<td>19</td>
</tr>
<tr>
<td>2.4 Subroutine DISP</td>
<td>21</td>
</tr>
<tr>
<td>2.5 Subroutine LINT</td>
<td>24</td>
</tr>
<tr>
<td>2.6 Subroutine HDIST</td>
<td>25</td>
</tr>
<tr>
<td>2.7 Subroutine COEF</td>
<td>26</td>
</tr>
<tr>
<td>2.8 Subroutine EFGEN</td>
<td>29</td>
</tr>
<tr>
<td>2.9 Subroutine GAIN</td>
<td>30</td>
</tr>
<tr>
<td>2.10 Subroutine FUNCT</td>
<td>31</td>
</tr>
<tr>
<td>2.11 Subroutine GENER</td>
<td>33</td>
</tr>
<tr>
<td>2.12 Subroutine GENS</td>
<td>33</td>
</tr>
<tr>
<td>2.13 Subroutine INIT</td>
<td>34</td>
</tr>
<tr>
<td>2.14 Subroutine INTEG</td>
<td>36</td>
</tr>
<tr>
<td>2.15 Subroutine LAGR</td>
<td>36</td>
</tr>
<tr>
<td>2.16 Subroutine MAMUL</td>
<td>37</td>
</tr>
<tr>
<td>2.17 Subroutine MATR</td>
<td>38</td>
</tr>
<tr>
<td>2.18 Subroutine MINV</td>
<td>39</td>
</tr>
<tr>
<td>2.19 Subroutine MODAL</td>
<td>40</td>
</tr>
<tr>
<td>2.20 Subroutine MPRD</td>
<td>42</td>
</tr>
<tr>
<td>2.21 Subroutine PORE</td>
<td>43</td>
</tr>
<tr>
<td>2.22 Subroutine RROOT</td>
<td>46</td>
</tr>
<tr>
<td>2.23 Subroutine SETL</td>
<td>46</td>
</tr>
<tr>
<td>2.24 Subroutine STAB</td>
<td>48</td>
</tr>
<tr>
<td>2.25 Subroutine VARYR</td>
<td>50</td>
</tr>
<tr>
<td>3. Input Data Cards</td>
<td>52</td>
</tr>
<tr>
<td>4. Sample Problems</td>
<td>68</td>
</tr>
<tr>
<td>4.1 Sample Problem for Settlement Analysis</td>
<td>69</td>
</tr>
<tr>
<td>(Sample Problem #1)</td>
<td></td>
</tr>
<tr>
<td>4.2 Sample Design Problem (Sample Problem #2)</td>
<td>69</td>
</tr>
<tr>
<td>4.3 Summary and Conclusions</td>
<td>70</td>
</tr>
<tr>
<td>References</td>
<td>74</td>
</tr>
<tr>
<td>Appendix A: Listing of the Program</td>
<td>117</td>
</tr>
</tbody>
</table>
1. Introduction

"Modified Sand" is a computer program for the general analysis of an embankment foundation on a soft soil with consideration of vertical and horizontal consolidation without any sand drain installations. This is a modified version of the program SAND (Krizek and Krugmann, 1972) that can be used to analyze this problem with consideration of vertical sand drains. The solution technique in the modified program remains the same as in the original program.

1.1 Capabilities of Modified Sand

This program optimizes the rate at which a specific highway embankment can be constructed on soft soil. This problem involves the computation of stresses and pore pressures in the subsoil, the dissipation of these pore pressures, the corresponding increase in shear resistance and stability of the embankment.

The embankment load which is assumed to act vertically, induces pore pressures in the subsoil which are computed using Theory of Elasticity and Skempton's pore pressure parameters (A, B). These pore pressures dissipate according to three dimensional consolidation theory which takes into account the effect of gas and variable soil parameters. The solution of consolidation equations are solved numerically by treating it as an eigenvalue problem. As the pore water pressure dissipates the effective stresses in the subsoil will increase giving a simultaneous
increase in shearing resistance. Settlements are computed from the dissipated pore pressures.

There are two options available.

**Option 1:** (For ISP = 0) -- The program determines the times at which each lift whose resulting shapes are input can be constructed without exceeding the bearing capacity of the subsoil and after a specified fraction of a reference settlement has occurred. Settlements and average degree of consolidation are output for specified points in a graphical form.

**Option 2:** (ISP = 1) -- The lifts and times of lift application are input and the program determines the dissipation of pore pressures and settlements for specified points. Bearing capacity of the foundation soil is not analyzed.

2. Description of the Program

Described in the subsequent sections is the set of computer programs which can be used to analyze an embankment foundation on a soft soil with consideration of vertical and horizontal consolidation. Individual routines, consisting of main program "Modified Sand" are written in FORTRAN 77. Program listing is attached in Appendix A. The programs have been tested in IBM PC XT and VAX 11/780 at Purdue University.

Each subsection of the program is given below explaining the following:
1. Purpose of the program.

2. Usage of the program.

3. Block names.

4. Description of parameters.

5. Method of solution or calculation.

6. Subroutines required.

In addition a list of sequence of input of data into the main program is given.

Two sample problems which illustrate most of the special features of the programs and solutions to these problems are attached.

2.1 The Main Program Modified SAND

Purpose of the Programs:

SAND -- To analyze an embankment foundation on a soft soil for stability and/or settlements and consolidation behavior with consideration of horizontal and vertical drainage.

Block Names and Lengths:

SAPOD/ IOUTP,W,HH,GLOAD,CLOAD,NARC,NRAD; length: 7 words
SADI1/ LAYER,IBCV,MHE,M,N,IDC,NDR,ISUM; XET(41); length: 49 words
SADI2/ FIMPV,RC,RK,C,RO,RE,TA,ISP,IVAR; length: 29 words
SACSE/ ROC,ROCL,SVM,P,PC,PLOG,PO,PCO,IAV,IK,ISAT,AAV,AAH; length: 54 words
SAC01/ AVOC,KVO,KHO,EOPUS,PU,SKHM,SKVM,CCC,NNN,ICOEF; length: 10 words
SAC02/ PCV(10),CVIN(10),PCH(10),CHIN(10),ICV,KOUNT,HF; length: 43 words
SADET/ XSTAB(51),YSTAB(11),DX,DY,YWM,TGPHI; length: 66 words

These blocks are only defined in program SAND

SAPOD is needed in subroutines DETFS,DISP,INIT,PORE, and VARYR
SADI1 and SADI2 are needed in subroutine DISP
SACSE is needed in subroutines COEF and SETL
SAC01 and SAC02 are needed in subroutine COEF
SADET is needed in subroutine DETFS

Description of Parameters

On the following pages are described the parameters which are input by the user; quantities listed in labelled COMMON blocks are given in the respective subroutines. This list is given in alphabetical order. Section (3) provides the list where data cards of the parameters appear in the sequence in which they are needed in the programs. The asterisk * refers to a note at the end of the list.

A - Skempton's pore pressure coefficient
AVO - constant coefficient of compressibility to be used in the settlement computations; in the case of two layers, AVO applies to the upper layer; dimension; ft/lb
AX(I) - subinterval limits to be input as decimal fractions of reference value W; I=1,NI where NI < 5
B - Skempton's pore pressure coefficient

BLANK - symbol to be used in the resulting plots

C - fraction of the reference load at time TA, which is applied at time equal to zero; may vary between 0.0 and 1.0

CC - compression index; negative slope of the void ratio versus effective stress curve (virgin part of the curve); in the case of two layers, CC is the compression index of the upper layer

CLOAD - undrained strength of the embankment oil dimension - psf

CO(I) - initial undrained strength of the subsoil; I=1, NC where NC < MYE < 11. If NC < MYE, Lagrangean interpolation is used to compute the undrained strengths at MYE equally spaced depths. If NC=MYE, the input values must be provided at equally spaced points where I=1 and I=MYE coincide with the surface and the bottom of the compressible layer, respectively; dimension - psf

COUNT* - marker to indicate the last residual pore water pressure data card; this parameter is zero on all residual pore water pressure data cards, except on the last card, where it must take value different from zero

CP(I) - (c/\bar{p})-ratios of the subsoil; I=1, NC; see remarks under CO(I)

CH - constant coefficient of consolidation in horizontal direction; in case of two layers, CH applies to the upper layer; dimension - ft^2/day

CHIN(I) - variable coefficients of consolidation in horizontal direction; I=1, ICV < 10; stress-dependent coefficients of consolidation in horizontal direction are obtained within subroutine COEF by interpolation between the CHIN-values; dimension - ft^2/day
CV - constant coefficient of consolidation in the vertical direction; in case of two layers, CV applies to the upper layer; dimension - ft$^2$/day

CVIN(I) - variable coefficients of consolidation in the vertical direction; I=1, ICV < 10; stress-dependent coefficients of consolidation in the vertical direction are obtained within subroutine COEF by interpolation between the CVIN-values; dimension - ft$^2$/day

DMAX - maximum step size to be used in the variation of XC and YC in the search procedure for the minimum factor of safety; dimension - feet

DMIN - minimum step size to be used in the variation of XC and YC in the search procedure for the minimum factor of safety; dimension - feet

EO - initial void ratio; in the case of two layers, EO applies to the upper layer

FSI - factor of safety which is required at the time of application of a new load

GAMMA - effective unit weight of the subsoil, constant over the thickness of the compressible layer; this value is needed in the settlement computations using the compression index; if GAMMA=0, input of MYE effective overburden stresses at equally spaced depths must be input; dimension - pcf

GLOAD - unit weight of the embankment soil, dimension - pcf

GRID - symbol to be used in the resulting plots to mark the 10% coordinates; proposed to be the letter I

H - thickness of the compressible layer; if H=0, the program is terminated;
if H=99, a branch is made to the beginning of the programs; dimension - feet

**HC**  - Henry's constant of gas solubility, 
  HC=0.020 for atmospheric air, 
  HC=0.029 for methane, HC=2.84 for hydrogen sulfide (at 68°F)

**HI**  - thickness of the "impedance layer" underlying the compressible soil; 
  the "impedance layer" must have a freely draining lower surface, a 
  coefficient of compressibility which is negligibly small compared to that 
  of the consolidating soil, and a 
  permeability of the same order of magnitude as that of the consolidation soil; dimensions - feet

**IAB**  - identifier where - 
  IAB=0 - Skempton's pore pressure coefficients A and B as defined for the last load 
           are also used to compute the pore water pressures due to the load addition 
  IAB=1 - redefine A and B

**IAV**  - identifier where - 
  IAV=0 - use a constant coefficient of compressibility in the settlement computations 
  IAV=1 - use the compression indices in the settlement computations

**IBCV**  - identifier where - 
  IBCV=1 - impeded vertical drainage at the bottom of the consolidating layer 
  IBCV=2 - free vertical drainage at the bottom of the consolidating layer 
  IBCV=3 - no vertical drainage at the bottom of the consolidating layer

**ICV**  - number of data pairs (PCV(I),CVIN(I)) and (PCR(I),CRIN(I)) through which Lagrangean interpolation polynomials are passed; 
          0 < ICV < 10

**IDEN(I)**  - identifier corresponding to the I-th load step, where
IDEN(I)<0 - the excess pore water pressures due to the first load step are set equal to the input residual pore water pressures

IDEN(I)=0 - the excess pore water pressures due to the I-th load are computed by means of subroutine PORE

IDEN(I)=1 - the excess pore water pressures due to the I-th load are set equal to those computed for the reference load. Note, that this requires that Skempton's coefficients A and B are identical in both cases.

IDEN(I)<0 allows the check of an existing installation for which the excess pore water pressures just after load application are known from field measurements; I=1, NL

IEND - number of horizontal coordinates XT; IEND is computed, if ISP=0; IEND < 20

IK - identifier where
IK=0 - constant coefficients of permeability
IK=1 - the radial and vertical coefficients are variable; the void ratio versus the logarithm of the coefficient of permeability is a straight line

IRP - identifier where
IRP=0 - no residual pore water pressures are input
IRP=1 - residual pore water pressures at points (W'XT, H'YE) are input columnwise
IRP=2 - residual pore water pressures at arbitrary points are input

ISAT - identifier where
ISP=0 - settlements, the process of consolidation, and the stability are analyzed; in program SAND, the times at which a new load step can be applied are determined;

ISP=1 - settlements and the process of consolidation are analyzed, and the times of load application are required as input parameters in program SAND; ISP=1 also requires the output of the time-dependent pore water pressures at MRE'MYE
points of the solution domain of the sand drain installations with axes at the user-defined locations XT

**ITBL** - number of times TB(I), defined in a DATA-statement, for which the pore water pressures and settlements are determined; times TB always start at the time of application of a new load step in SAND; ITBL < 45

**IVAR** - identifier where
- IVAR=0 - use constant coefficients of consolidation
- IVAR=1 - use variable coefficients of consolidation which are obtained either by interpolation between CHIN(I) and CVIN(I) or by varying the coefficient of compressibility and/or the coefficients of permeability

**JND** - number of points for which output is required; JND < 10

**JSP(I)** - indices of the JND points for which output is required; I=1, JND < 10; the output is for points XE (JSP(I)), where XE and MX equally spaced coordinates between and including the limits W'AX(1) and W'AX(NI); for example, specification of JSP(1)=1 and JSP(JND)=MX causes the output of information at the limits W'AX(1) and W'AX(NI), respectively

**KHO** - initial coefficient of permeability in the horizontal direction; dimension - ft/day

**LAYER** - number which indicates the location of a layer interface; LAYER=KK means that the layer interface is located at a depth below ground surface which is equal to Y=H*(KK-1)/(MYE-1); if only one layer is to be considered, set LAYER=0; the program requires that 3 < LAYER < (MYE-3); LAYER causes a layer interface to be considered in the consolidation and the settlement analyses only

**LND** - number of weeks to be plotted on the time axis of the output figures

**MINP** - number of points defining the contour of the embankment load; MINP < 20
MHE - number of equally spaced points in the horizontal direction; MHE < 40

MX - number of equally spaced points XE in the horizontal direction between the limits AX(1) and AX(NI); MX < 51

MXT(I) - number of unequally spaced points XT between the consecutive limits AX(I) and AX(I+1); I=1, (NI-1) < 4; maximum value of any MXT(I) must be values MXT(I) must not exceed IEND=20

MYE - number of equally spaced points in the vertical direction, including the surface and the bottom of the compressible layer; MYE < 12

NC - number of initial undrained shear strengths, CO(I), and (c/p)-ratios, CP(I); NC < MYE < 11

NI - number of interval limits AX(I); NI < 5

NL - number of load steps; NL < 6

NRAD - number of trial arcs to be used with each trial center (XC,YC) in the stability analysis; NRAD > 1

NS - number of load strips used to approximate the actual embankment load; NS < 20

P(I) - present overburden effective stresses at MYE equally spaced depths, including the surface; I=1, MYE < 12; dimension - psf

PC(I) - preconsolidation stresses at MYE equally spaced depths, including the surface; I=1, MYE < 12; dimension - psf

PCH(I) - effective stresses at which the horizontal coefficients of consolidation, CHIN(I), are defined; I=1, ICV < 10; dimension - psf

PCV(I) - effective stresses at which the vertical coefficients of consolidation, CVIN(I), are defined; I=1, ICV < 10; dimension - psf
PU - initial pore gas pressure; if PU is not defined during input, it is set equal to the sum of the atmospheric pressure plus one-half the thickness of the compressible layer times the unit weight of water; dimension - psf

RAV - coefficient of compressibility of the lower layer divided by that of the upper layer

RC - vertical coefficient of consolidation of the lower layer divided by that of the upper layer

RCC - virgin compression index of the lower layer divided by that of the upper layer

REO - initial void ratio of the lower layer divided by that of the upper layer

RK - vertical coefficient of permeability of the lower layer divided by that of the upper layer note, if RC and RAV are specified, RK=RC*RAV*(1+EO)/(1+EO*REO)

RKV - vertical coefficient of permeability of the consolidating soil divided by that of the underlying impedance layer

ROC - recompression index divided by the virgin compression index; in case of two layers, ROC applies to the upper layer

ROCL - recompression index of the lower layer divided by the virgin compression index of the upper layer

S - degree of saturation to be input as a decimal fraction

SKH - slope of the void ratio versus the logarithm of the horizontal coefficient of permeability curve

SKV - slope of the void ratio versus the logarithm of the vertical coefficient of permeability curve

SPECS - fraction of the consolidation settlement due to the reference load; this settlement must have occurred before a new load is applied (program SAND)

SPECU - when the non-dissipated average pore water pressures become less than 5% of the total average
pore water pressures existing just after application of the last load at IEND*SPECU points XT, subsequent loads are disregarded; the rationale for this procedure is that no significant increase in strength and/or settlement can be expected after an average degree of consolidation of 95% has been reached under the applied load at a number of points; in selecting the magnitude of SPECU, which is input as a decimal fraction, it should be noted that the degrees of consolidation in the case of constant coefficients of consolidation will be the same for different points XT, as long as the drainage boundary conditions are the same.

STAR - symbol to be used in the resulting plots to mark the coordinate axes; proposed to be the asterisk *

SYMB(I) - symbols to be used in the resulting plots to present points of the computed curves; the letters U, C, 0, and T are proposed for the average degree of consolidation, the consolidation settlement, the initial, and the total settlement versus time curves, respectively. It should be noted that T plots on top of 0, which plots on top of C, which plots on top of U; this means, that only T will show, when the four values are identical; I=1,2 in the case of complete saturation, and I=1,4 in the case of partial saturation. User can use any other letters as symbols.

TA - available construction time; in SAND, this is the time at which the final load must have been applied; dimension - days

TGPHI - tangent of the angle of internal friction of the drainage blanket, if there is one

TL(I) - times of load application in the case where ISP=1; I=1, NL < 6; dimension - days

TMIN - time which must have passed after a load application before the first stability analysis is made to determine, whether the next load can be applied; dimension - days

U* - residual pore water pressure; dimension - psf
W  - reference value in the horizontal direction;  
    dimension - feet

X*  - horizontal distance from the center line at which  
    the residual pore water pressure is known;  
    dimension - feet

XC  - X-coordinate of the center of the first trial arc;  
    if XC=0 is input, the programs select a starting  
    value; dimension - feet

XINP(I)  - X-coordinates of the points defining the embank-  
    ment contour; I=1, MINP < 20; dimension - feet

XT(I)  - X-coordinates of the points at which the settle-  
    ments and the consolidation behavior are deter-  
    mined; if ISP=0, XT(I) are computed for I=1  
    through MXT(J); if ISP=1, XT(I), I=1, IEND < 20  
    are input as fractions of W

Y*  - vertical distance below the ground surface at  
    which the residual pore water pressure U is known;  
    positive downward; dimension - feet

YC  - Y-coordinate of the center of the first trial arc;  
    positive upward; dimension - feet

YINP(I)  - Y-coordinates of the points defining the embank-  
    ment contour; positive upward; I=1, MINP < 20;  
    dimension - feet

YWM  - thickness of a drainage blanket placed on the  
    surface of the compressible soil layer; dimension  
    - feet

ZZ  - distance between the maximum YINP(I) and the  
    minimum value YC permissible in the stability  
    analyses; dimension - feet

* The residual pore water pressures are first arranged such  
  that points having the same X-coordinate are grouped in the  
  order of ascending Y-coordinates. Data sets are then in-  
  put in the order of ascending X, whereby the last card is  
  identified by COUNT # 0.
Method

The programs facilitate the analysis of an embankment foundation on a soft, compressible soil layer, which is underlain by a firm stratum. The approach involves the consideration of the following problems: (a) stress and pore pressure distribution within the soft layer due to a symmetrical vertically acting embankment load at the surface, (b) the dissipation of excess pore water pressures subject to different flow conditions including horizontal flow, (c) the computation of settlements, and (d) the stability of the embankment-subsoil system with consideration of the gain in shear strength as consolidation proceeds.

The programs are designed to solve several cases during the same program execution, wherefore some computations are performed before data for a specific case are input. To save computer time and storage, computations are only done for a limited number of locations in the horizontal direction, and information at intermediate points is obtained by interpolation.

PROGRAM SAND -- After computation of the pore water pressures and settlements due to a reference load, which in most cases will be identical with the final load, essentially two options are available by means of index ISP. If ISP=0, the embankment contours of the different load steps are input and the program determines the times at which new load steps can be applied. The criteria incorporated into the programs are: (1) a defined portion of the reference settlement at the point closest to the center line of the embankment must have occurred, and/or
(2) a specified factor of safety must be assured at the time of a new load application. To avoid unnecessarily numerous stability analyses, a time $T_{MIN}$ measured from the last load application can be defined, and stability analyses are not performed for times less than $T_{MIN}$, although settlements and degrees of consolidation are computed. For the same reason, the program contains the restriction that all subsequent load steps are disregarded if 95% consolidation has occurred at a specified number of points under the acting load. The rationale is that only a minor increase in strength and settlements can be expected due to dissipation of the remaining excess pore water pressures, and the times required will likely be prohibitive.

If ISP=1, the different load contours, as well as the times of load application, must be input, and the program analyzes the consolidation process and the settlements without performing any stability analyses. Use of ISP=1 also produces the output of the pore water pressures at MYE*IEND points of a vertical cut through the locations $XT^W$.

The computed information is first stored internally on two internal files for each step of load. A total of 6 steps of loads are allowed.

The program can handle analysis of multiple embankments in a single run. This is done by putting $H = 99$ at the end of the data cards for the previous embankment. On the other hand by specifying $H = 0$ the program is terminated.
The output, in addition to that given for ISP*0, includes the excess pore water pressures at MYE*JND points of a vertical section through the subsoil.

Remarks

The average degree of consolidation is defined in the programs as the integral over the dissipated pore water pressures divided by the integral over the excess pore water pressure build-up under the reference load.

The increase in the effective stresses at the time of load application in the case of a partially saturated soil is assumed to be equal to the difference between the pore water pressures obtained for $B=1.0$ and $B<1.0$, where $B$ is Skempton's pore pressure coefficient.

To account for the fact that the swelling index is normally considerably smaller than the compression index, negative pore water pressures, which might result after surcharge removal, are neglected in program SAND.

Subroutines Required

COEF (UAVD,UAVE,OMEGA,PHI,LI,IL,OMED,PHID,NN)
DISP (U,LI,OMEGA,PHI,T,UAVE,LIFT,MYE,IEND,XT,SV)
GAIN (UA,R,SU,MYE,MXT,MXE,MX,NIM,CO,CP,III)
GENS (S,M)
INIT (XINP,YINP,MINP,XC,YC,YY,ZZ,DMIN)
LAGR (X,Y,M,JST,XX,YY,N)
2.2 Subroutine APROX

Purpose

To approximate the embankment contour by a number of strips of constant thickness

Usage

CALL APROX (X,Y,MN,N,D)

Block Names

POAPI/ALPHA(30), L

Description of Parameters

X,Y - coordinates of the points defining the embankment contour; must be provided such that X(1)=0<X(2)<...<X(MN)

MN - number of points X,Y; MN < 20

N - number of approximating strips

D - thickness of the approximating strip

ALPHA - returned lengths of the strips

L - number of values ALPHA, L < 30

Statement Functions Required

CONK(K0,SKM);VARK(K0,SKM);PSI(AA,K)
2.3 Subroutine DETFS

Purpose

To determine the factor of safety of an embankment resting on a soft subsoil.

Usage

CALL DETFS(XC,YC,R,XINP,YINP,MINP,MX,MYE,SU,FS)

Block Names

INDET/RHO(19),TAU(19),PSI(19)
SAPOD/IOUTP*;W*;H*;GLOAD,CLOAD,NARC,NRAD*
SADET/XSTAB(51),YSTAB(11),DX,DY,YWM,TGPHI
* Parameters marked by an asterisk are not needed in this subroutine

Description of Parameters

XC  - X-coordinate of the center of the circular slip surface
YC  - corresponding Y-coordinate
R   - radius of the circular slip surface
XINP - X-coordinates of the points defining the embankment contour
YINP - corresponding Y-coordinates
MINP - number of points (XINP,YINP)
MX   - number of equally spaced grid points in the X-direction
Method

A total stress analysis is performed to evaluate the factor of safety of an embankment which consists of cohesive soil and a cohesionless drainage blanket. The undrained strengths of the subsoil are input at \( MX*MYE \) grid points, and the strength available along the portion of the circular slip surface that passes through the subsoil is obtained at the centers of \( 2*NARC \) subarcs.
by interpolation between the strengths SU at adjacent grid points. Resisting the driving moments are first computed with the assumption that the embankment consists entirely of frictionless soil. The so-obtained ratio of moments is then used as the initial estimate in the iteration for the correct factor of safety, in which the drainage blanket is considered.

Statement Functions Required

\[ \text{FUNA} (A,B), \text{FUNB} (B), \text{FUNC} (A,B,C) \]

Remarks

The coordinates YINP, YC, YWM and PSI are positive upward, wherein YSTAB is positive downward with the coordinate origin at the surface of the soft layer.

2.4 Subroutine DISP

Purpose

To determine the excess pore water pressures at arbitrary times for step loading conditions.

Usage

\[ \text{CALL DISP}(U,LI,OMEGA,PHI,T,UAVE,LIFT,MYE,IEND,XT,SV) \]

Block Names

\[ \text{SAPOD/IOUTP,W*,H*,GLOAD*,CLOAD*,NARC*,NRAD*;} \]
\[ \text{SADII/LAYER,IBCV,MHE,M,N,IDC,NDR,ISUM,XET(41);} \]
SADI2/FIMPV,RC,RK,C,RO,RE,TA,ISP,IVAR;
* parameters marked by an asterisk are not needed in this subroutine

Description of Parameters

U    - pore water pressures to be determined; for LI=1,5,6 this vector contains the additional pore water pressures for the new load, when subroutine DISP is called

LI=1 - determines vectors A and B for the load addition

LI=2 - determines the pore water pressures due to step-wise constant loads

LI=3 - determines vectors A and B for times between load applications in the case where the "consolidation factor" is variable

LI=5 - first lift; first execution of subroutine DISP

OMEGA - "consolidation factors" for radial flow; product of the gas factor and the radial coefficient of consolidation

PHI - "consolidation factors" for vertical flow; product of the gas factor and the vertical coefficient of consolidation

T    - time

UAVE - average pore water pressures

LIFT - number of lifts applied at and before time T

MYE  - number of points equally spaced in the vertical direction at which the pore water pressures are computed

MHE  - number of points equally spaced in the horizontal direction at which the pore water pressures are computed < 40

IEND - number of elements in vectors OMEGA, PHI, UAVE and XT
XT - points in the horizontal direction for which OMEGA and PHI are input and UAVE is computed

SV - mathematical molecule of the extended Simpson's or trapezoidal rules in the vertical direction

IOUTP - logical output unit

LAYER - index indicating the depth of a layer interface; LAYER > 3

IBCV=1 - vertical drainage; impeded drainage at the lower boundary surface

IBCV=2 - vertical drainage; free drainage at the lower boundary surface

IBCV=3 - vertical drainage; no drainage at the lower boundary surface

M - number of eigenvalues for the vertical problem

N - number of eigenvalues for the horizontal problem

IDC=1 - vertical flow only at all points XT

IDC=2 - vertical plus horizontal flow at all points XT

ISUM - number of elements of vector U

FIMPV - "impedance factor" for vertical flow; FIMPV = (RKV*HI/DY) / (1.+RKV*HI/DY), as defined in SAND

RC - ratio of the vertical coefficients of consolidation of the lower and upper layer

RK - ratio of the vertical coefficients of permeability of the lower and upper layer

ISP=1 - compute and print the pore water pressures at all MYE*MRE points of the solution domain for IEND locations XT; return the averages taken at MYE depths over the circular area of influence as vector U; return the overall average at IEND locations XT as vector UAVE

ISP=0 - suppress the printing
IVAR=0  - constant "consolidation factors"
IVAR=1  - variable "consolidation factors"

Method

The consolidation problem is treated as an eigenvalue problem.

Subroutines Required

EFGEN(PSI,T,EIG,IVAR,MM,NN,D,LI)
MAMUL(A,D,B,C,N,IS,II)
MODAL(LAYER,IBC,N,FIMP,RC,RK,XO,XE,EIG,X,XI,F)
MPRD(A,B,R,N,M,L,IAS,IBS,IRS)

Remarks

Storage reservations are made to account for IEND < 40 and a maximum of 6 step loads.

2.5 Subroutine LINT

Purpose

To interpolate between arbitrarily spaced data points by use of interpolation or extrapolation.

Usage

Call LINT(X,Y,N1,M,XX,YY,N)
Description of Parameters

\( X \) - vector of arguments for which the values of the function are interpolated

\( Y \) - resulting vector of interpolated values of the function

\( N_1 \) - number of arguments in \( X \)

\( M \) - index of the last value of \( Y \)

\( XX \) - vector of arguments for which the values of the function are known

\( YY \) - vector of known values of the function

\( N \) - number of arguments in \( XX \)

2.6 Subroutine HDIST

Purpose

To calculate the horizontal distance from the CL to the point where the pore pressure is 0.1% of the maximum pore pressure under the embankment.

Usage

Call HDIST(UB,XT,IEND,ICV,CHIN,DXSQ,AAH,MHE,W, XET,IPOR,HF,MYE,POR)

Description of Parameters

\( UB \) - pore pressure at (MYE*IEND) points under the embankment

\( XT \) - X-coordinate of the points at which the settlements and consolidation behavior are determined

\( IEND \) - number of horizontal coordinates XT
ICV - number of data pairs (PCV(I),CVIN(I)) and (PCH(I),CHIN(I)) through which Lagrangian interpolation polynomials are passed.

CHIN - variable coefficient of consolidation in horizontal direction.

DXSQ - \((\Delta H^2)\)

AAH - \((1. + E0)/(\gamma W x (\Delta H)^2)\)

MHE - number of horizontal grid points.

W - Reference width.

XET - X-coordinates of the equidistant points in the horizontal direction.

IPOR - indicator to specify the value of POR:
   - 1, POR is specified by the user.
   - 0, program evaluates POR.

HF - 1, If horizontal flow is considered.
   - 0, If no horizontal flow is allowed.

MYE - number of points in the vertical direction.

POR - horizontal drainage distance/\([XT(IEND) \times W]\)

2.7 Subroutine COEF

Purpose

To determine the gas factor and the coefficients of consolidation.

Usage

COEF-UAUD,UAUE,OMEGA PHI,LI,IL,OMED,PHID,NN]

Block Names

SACSE/ROC,ROCL,SVM,P,PC,PLOQ,PO,PCO,LAV,IK,ISAT,AAV,AAH
SAC01/AVO,KVO,KHO,EOPUS,PU,SKHM,SKvm,CCC,NNN,ICOEFF
SAC02/PCV(10),CXIN(10),PCR(10),CHIN(10),ICV,KOUNT,HF
Description of Parameters

UAVD - average pore pressure before consolidation
UAVE - average pore pressure at some time after consolidation
OMEGA - consolidation factors of horizontal flow, product of the gas factor and the horizontal coefficient of consolidation
PHI - consolidation factor for vertical flow, product of the gas factor and the vertical coefficient of consolidation
IL - indicator
  =1 - calculate the parameters for vertical flow
  #1 - calculate the parameters for horizontal flow
LI - identifier; if LI=3, OMED and PHID are computed
NN - number of points where OMEGA, PHI, OMED and PHID are required
OMED - difference between the radial consolidation factor computed in a previous execution of this subroutine and the value computed in this execution of the subroutine
PHID - difference between the vertical consolidation factor computed in a previous execution of this subroutine and the value computed in this execution of the subroutine
IEND - number of elements in arrays UAVD, UAVE, OMEGA, PHI, OMED, PHID
ROC - ratio between the recompression and the virgin compression indices
PO - average initial vertical effective stress
PCO - average preconsolidation stress
IAV=0 - constant coefficient of compressibility
IAV=1  - variable coefficient of compressibility
IK=0  - constant coefficient of permeability
IK=1  - variable coefficient of permeability
ISAT=0  - 100% saturation
ISAT=1  - partial saturation
AAV  - factor defined in program SAND; AAV=(1+E0)/(62.43*DY²)
AAH  - factor defined in program SAND; AAH=(1+E0)/(62.43*DH²)
AVO  - initial or constant coefficient of compressibility
KVO  - initial coefficient of permeability in the vertical direction
KRO  - initial coefficient of permeability in the radial direction
EOPUS  - factor defined in program SAND; EOPUS=E0*PU*(1-S)*(1-HC)
PU  - initial pore gas pressure
SKVM  - factor defined in program SAND; SKVM=CC/SKV, if IAV=1 and SKVM=2.3026*AVO/SKV, if IAV=0
SKHM  - factor defined in program SAND; SKHM=CC/SKH, if IAV=1 and SKHM=2.3026*AVO/SKH, if IAV=0
CCC  - compression index times 0.4343
NNN  - number of locations with radial and vertical drainage conditions
ICOEF=1  - IK=0, IAV=0 or IAV=1
ICOEF=2  - IK=1, IAV=0
ICOEF=3  - IK=1, IAV=1
ICOEF=4  - the coefficient of consolidation is obtained by interpolation
PCV - effective stresses for which the vertical coefficients of consolidation are input
CVIN - vertical coefficients of consolidation at PCV
PCH - effective stresses for which the radial coefficients of consolidation are input
CHIN - radial coefficients of consolidation at PCH
ICV - number of PCV, CVIN, PCR, and CRIN; ICV < 10
KOUNT=0 - second or subsequent executions of this subroutine
KOUNT=1 - first use of this subroutine

Method and Reference

Depending on the values of the indices ISAT, IK, IAV, and ICOEF, the values of the "consolidation factors" for radial and vertical flow are determined for the average increases in effective stresses (UAVD-UAVE) at IEND locations. Relationships considered include: (bi-) linear void ratio versus logarithm of effective stress or constant coefficient of compressibility; linear void ratio versus logarithm of coefficient of permeability; and arbitrary coefficient of consolidation versus effective stress relationships.

2.8 Subroutine EFGEN

Purpose

To generate the time-dependent matrix D.

Usage

CALL EFGEN(PSI, T, EIG, IVAR, MM, NN, D, LI)
Description of Parameters

PSI - vector containing MM "consolidation factors"
T - time at which diagonal matrix D is computed
EIG - vector containing the eigenvalues
IVAR=0 - constant "consolidation factor"; PSI consists of one element only
IVAR=1 - variable "consolidation factor"; PSI consists of MM elements
MM - number of elements PSI (in most other routines, this parameter is called IEND)
NN - number of eigenvalues
D - diagonal matrix to be determined

Method

The elements of the diagonal matrix D are given by exp (PSI(J)*EIG(I)*T), wherefore D has a total of MM*NN elements. However, if IVAR=0, D(K)=D(K+NN)=...=D(K+(MM-1)*NN).

2.9 Subroutine GAIN

Purpose

To determine the gain in shear strength.

Usage

CALL GAIN(UA,R,SU,MYE,MXT,MXE,MX,NIM,CO,CP,III)
Description of Parameters

UA - vector of dissipated pore water pressures at points (XT,XE)

R - auxiliary matrix necessary to compute the dissipated pore water pressures at points (XE,YE) from a knowledge of those at points (XT,YE)

SU - resultant undrained strengths at points (XE,YE)

MYE - number of equally spaced points in vertical direction

MXT(I) - number of points XT between the interval limits AX(I) and AX(I+1)

MXE(I) - number of points XE between the interval limits AX(I) and AX(I+1)

MX - sum of MXE(I) for I=1,NIM

NIM - number of subintervals

CO - vector containing MYE undrained initial shear strength values

CP - vector containing MYE (c/\bar{p})-ratios

III=1 - all elements of array UA are assumed to be equal to zero

III=0 - some or all elements of array UA differ from zero

Method

The strength values SU are obtained as the sum of the initial shear strengths plus the products of the (c/\bar{p})-ratios and the dissipated pore water pressures.

2.10 Subroutine FUNCT
Purpose

This subroutine computes the values of the integrands for the argument theta.

Usage

CALL FUNCT(THETA,ETA,K,SIGX,SIGY,TAU)

Block Names

POFUN/Q(258),ETHST(258)

Description of Parameters

K      - index necessary to select the proper quantities Q and ETH, which have been precomputed for the same argument THETA
SIGX   - value of the integrand of the equation for the horizontal normal stress
SIGY   - value of the integrand of the equation for the vertical normal stress
TAU    - value of the integrand of the equation for the shear stress
Q      - precomputed vector whose elements are equal to the sum of \((\sin \theta)/\theta\)
ETH    - precomputed vector whose elements are equal to \(\exp(\theta)\)

Method

The subroutine makes use of the fact that the hyperbolic sine and cosine functions can be expressed in terms of the exponential function.
2.11 Subroutine GENER

Purpose

To determine the coefficients and the roots of the characteristic equation.

Usage

CALL GENER(P,F,X,N)

Description of Parameters

P - tridiagonal matrix whose lower off-diagonal elements are equal to -1.0
F - auxiliary matrix used during the computations
X - roots of the characteristic equation; these are the eigenvalues
N - degree of the characteristic equation

Subroutines Required

RROOT (A,X,N)

2.12 Subroutine GENS

Purpose

To generate the mathematical molecules which are used in a numerical integration.

Usage

CALL GENS(S,M)
Description of Parameters

S  - resulting mathematical molecule
M  - number of pivotal points

Method

For the case of vertical flow, the elements of vector S are either computed by the extended Simpson rule or the extended trapezoidal rule assuming equal spacing; the use of Simpson's rule requires that M be an odd number.

2.13 Subroutine INIT

Purpose

To select starting values for the stability analysis and define three vectors which are repeatedly used in subroutine DETFS.

Usage

CALL INIT(XINP,YINP,MINP,XC,YC,YY,ZZ,DMIN)

Block Names

SAPOD/IOUTP*,W*,H,GLOAD*,CLOAD*,NARC*,NRAD*;
INDET/RHO(19),TAU(19),PSI(19);
* parameters marked by * are not needed in this routine
Description of Parameters

XINP - X-coordinates of the points defining the embankment contour

YINP - Y-coordinates of the points defining the embankment contour

MINP - number of points (XINP,YINP)

XC - X-coordinate of the center of the first trial slip surface

YC - corresponding Y-coordinate

YY - minimum permissible value for YC

ZZ - difference between the maximum YINP-value and YY

DMIN - minimum increment to be used in the direct search procedure

H - thickness of the compressible layer

RHO - slopes of the lines which connect consecutive points (XINP,YINP)

TAU - TAU=1+RHO**2

PSI - Y-value at X=0 for the lines connecting consecutive points (XINP,YINP)

Method

When the center of the first trial failure arc is not input from SAND, YC is set equal to YY and XC is defined as one-half of the sum of the X-values obtained when two circles with radius R=YY+H pass through (XINP(I),YINP(I)) and the toe of the embankment, respectively. The vectors RHO, TAU, and PSI are computed once for repeated use in subroutine DETFS.
2.14 Subroutine INTEG

Purpose

To compute approximate values of the stress integrals between the limits $B$ and infinity.

Usage

CALL INTEG(ETA, XI, B, AR)

Block Names

PCAPI/ALPHA(30), L;

Description of Parameters

ETA - Y-coordinate divided by the thickness of the compressible layer

XI - X-coordinate divided by the thickness of the compressible layer

B - lower integration limit

AR - resulting array with six integral values

ALPHA - length of the load strips divided by the thickness of the compressible layer

L - number of ALPHA's

2.15 Subroutine LAGR

Purpose

To interpolate between arbitrarily spaced data points by use of the Lagrangean polynomial.
**Usage**

CALL LAGR(X,Y,M,JST,XX,YY,N)

**Description of Parameters**

- **X** - vector of arguments for which the values of the function are interpolated
- **Y** - resulting vector of interpolated values of the function
- **X** - number of arguments X
- **JST** - index of the first value Y to be interpolated
- **XX** - vector of arguments for which the values of the function are known
- **YY** - vector of known values of the function
- **N** - number of arguments XX

**Method**

A Lagrangean polynomial of degree (N-1) is passed through the data points (XX,YY) and then evaluated for M arguments X. See, for example, the book by CARNAHAN, LUTHER, AND WILKES (1969).

**2.16 Subroutine MAMUL**

**Purpose**

To perform the matrix multiplication: (general matrix)*(diagonal matrix)*(column vector).
**Usage**

CALL MAMUL(A,D,B,C,N,IS,II)

**Description of Parameters**

A - general square matrix  
D - diagonal matrix  
B - column vector  
C - resulting column vector  
N - order of matrices A and D and length of vectors B and C  
IS - index of the first element of vector B  
II - index of the first element of matrix D, whose diagonal elements only are stored one-dimensionally

**Method**

The subroutine utilizes the fact that all matrices are stored one-dimensionally, so that the I-th element of vector C becomes

\[ C(I) = \sum_{k=1}^{N} A(I+K*N-N)B(IS-1+K)D(II-1+K) \]

2.17 Subroutine MATR
Purpose

To generate matrix XM, the elements of whose rows are equal to integer powers of the differences between the elements of vector XV and constant A.

Usage

CALL MATR(IS,IE,M,XV,A,XM)

Description of Parameters

IS - index of the first element of vector XV
IE - index of the last element of vector XV
M - number of rows of matrix XM
XV - vector with (IE-IS+1) elements
A - constant to be subtracted from all elements XV
XM - resulting M by (IE-IS+1) matrix

Method and Reference

Given the vector XV with elements
XV(IS),XV(IS+1),...,XV(IE), the M by (IE-IS+1) matrix is generated and stored one-dimensionally, such that XM (K+I*M-M) =(XV(IS+I-1)-A)**(K-1).

Program Length

45 words

2.18 Subroutine MINV
Purpose

To invert a general matrix.

Usage

CALL MINV(A,N,D)

Block Names and Lengths

None

Description of Parameters

A - input matrix destroyed in computation and replaced by the resultant inverse

N - order of matrix A; N < 25

D - resulting determinant

Method and Reference

The standard Gaub-Jordan method is used. This subroutine is a slightly modified version of subroutine MINV, as given in the IBM Application Program, 1130 Scientific Subroutine Package (1130-CM-02X), Programmer's Manual, Form H20-0252-0, White Plains, New York, 1966.

2.19 Subroutine MODAL
Purpose

To determine matrix P, its eigenvalues, the corresponding modal matrix, and the inverse of the modal matrix.

Usage

CALL MODAL(LAYER,IBC,N,FIMP,RC,RD,XO,XE,EIG,X,XI,F)

Description of Parameters

- **LAYER=1**  - radial drainage conditions
- **LAYER=2**  - vertical drainage conditions; homogeneous soil profile
- **LAYER > 3**  - vertical drainage conditions; two-layered soil profile with layer interface at YE(LAYER)
- **IBC=1**  - vertical flow; impeded drainage at the bottom
- **IBC=2**  - vertical flow; free drainage at the bottom
- **IBC=3**  - vertical flow; no drainage at the bottom
- **N**  - number of eigenvalues
- **FIMP**  - "impedance factor"; for vertical flow \((RKV*HI/DY)/(1+RKV*HI/DY)\)
- **RC**  - ratio of the vertical coefficients of consolidation of the lower and the upper layers
- **RK**  - ratio of the vertical coefficients of permeability of the lower and the upper layers
- **XO**  - lower boundary of the solution domain
- **XC**  - upper boundary of the solution domain
- **EIG**  - resultant eigenvalues
- **X**  - resultant modal matrix
XI - inverse of the resultant model matrix
F - auxiliary matrix

Method and Reference

For IBC=2 and IBC=3, the eigenvalues and the modal matrix can be computed directly for a homogeneous soil profile. In all other cases, the auxiliary matrix D and matrix P, whose eigenvalues are determined in subroutine GENER, must be generated before the modal matrix X can be set up. Finally, the inverse of the modal matrix is computed by use of subroutine MINV.

Subroutines Required

GENER(P,F,X,N)
MINV(A,N,D)

Remarks

The lower off-diagonal elements of matrix P, which are equal to -1.0, are not stored.

2.20 Subroutine MPRD

Purpose

To multiply two matrices to form a resultant matrix.

Usage

CALL MPRD(A,B,R,N,M,L,IAS,IBS,IRS)
Description of Parameters

A  - first input matrix
B  - second input matrix
R  - output matrix
N  - number of rows of matrices A and R
M  - number of columns of matrix A and number of rows of matrix B
L  - number of columns of matrices B and R
IAS - index of the first element of matrix A
IBS - index of the first element of matrix B
IRS - index of the first element of matrix R

Method

The M by L matrix B is premultiplied by the N by M matrix A and the result is stored in the N by L matrix R. The indices IAS, IBS, and IRS allow the multiplication of submatrices of A and B, and the product is stored as a submatrix of R.

Remarks

Matrix R cannot be in the same location as matrices A or B.

2.21 Subroutine PORE

Purpose

To compute the elastic stresses and pore water pressures within a layer of finite thickness for a symmetrical vertical load.
Usage

CALL PORE(XINP,YINP,M,NST,CX,IX,CY,IY,U,ABAR,BBAR)

Block Names

SAPOD/IOUTP,W,H,CLOAD,CLOAD*,NARC*,NRAD*
P0API/ALPHA(30),L;
POFUN/QST(129),ETHST(129);
* parameters marked by an asterisk are not needed in this subroutine

Description of Parameters

XINP - X-coordinates of the points defining the embankment contour

YINP - corresponding Y-coordinates

M - number of points (XINP,YINP)

NST - number of approximating load strips

CX - X-coordinates divided by the reference value W, for which the stresses are to be computed

IX - number of CX-values

CY - Y-coordinates divided by the thickness of the compressible layer H, for which the stresses are to be computed

IY - number of CY-values

U - resulting excess pore water pressures (IX*IY < 220 elements)

ABAR - Skempton's pore pressure coefficient A

BBAR - Skempton's pore pressure coefficient B

IOUTP - logical output unit
W  - reference length in X-direction
H  - thickness of the compressible layer
GLOAD  - unit weight of the embankment soil
ALPHA  - lengths of the load strips which approximate the actual embankment load
L  - number of values ALPHA
QST  - resulting vector whose elements are repeatedly used in subroutine FUNCT
ETH  - resulting vector whose elements are repeatedly used in subroutine FUNCT

Method and Reference

The total stresses within a compressible layer are computed by use of elastic theory for plane strain conditions and a symmetric vertical loading. Poisson's ratio is set equal to 0.5, and the underlying stratum is assumed to be rough and rigid. Because of the complex nature of the stress integrals, a numerical integration procedure, based on either Simpson's rule of Filon's formulae, has been chosen for their evaluation.

Subroutines Required

APROX(X,Y,MN,N,D)
FUNCT(THETA,ETA,K,SIGX,SIGY,TAU)
INTEG(ETA,XI,B,AR)

Remarks

The coordinates YINP are positive upward, whereas ETO is positive downward with the coordinate origin at the surface of the compressible layer. ETA is positive upward with the origin at the bottom of the compressible layer.
2.22 Subroutine RROOT

Purpose

To compute the real roots of the characteristic equation.

Usage

CALL RROOT(COF,XR,M)

Description of Parameters

COF - input vector containing the \((M+1)\) coefficients of the polynomial

XR - resulting \(M\) roots of the polynomial

M - degree of the polynomial

2.23 Subroutine SETL

Purpose

To compute settlements for constant or variable coefficients of compressibility.

Usage

CALL SETL(U,SETTL,IEND,KKK,MYE,F,FUP,FLO,KIAV)

Block Names and Lengths

SACSE/ROC,ROCL,SVM,P,PC,PLOG,PO*,PCO*,IAV*,IK*,ISAT*,AAV*,AAH
* parameters marked by an asterisk are not needed in this subroutine
### Description of Parameters

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>U</td>
<td>input vector of dissipated pore water pressures with ((\text{MYE} \times \text{IEND})) elements</td>
</tr>
<tr>
<td>SETTL</td>
<td>resulting vector of settlements</td>
</tr>
<tr>
<td>IEND</td>
<td>number of elements of SETTL</td>
</tr>
<tr>
<td>KKK</td>
<td>number of points in the upper layer in the vertical direction</td>
</tr>
<tr>
<td>MYE</td>
<td>total number of points in the vertical direction</td>
</tr>
<tr>
<td>F</td>
<td>multiplying factor; if (F=1.0), the consolidation settlements are computed; if (F=1/B), where (B) is Skempton's pore pressure parameter, total settlements are computed</td>
</tr>
<tr>
<td>FUP</td>
<td>parameter for the upper layer; contains the soil parameters</td>
</tr>
<tr>
<td>FLO</td>
<td>parameter for the lower layer; contains the soil parameters</td>
</tr>
<tr>
<td>KIAV=1</td>
<td>a constant coefficient of compressibility is used</td>
</tr>
<tr>
<td>KIAV=2</td>
<td>a variable coefficient of compressibility is used</td>
</tr>
<tr>
<td>ROC</td>
<td>ratio between the recompression and the virgin compression indices for the upper layer</td>
</tr>
<tr>
<td>ROCL</td>
<td>recompression index of the lower layer divided by the virgin compression index of the lower layer</td>
</tr>
<tr>
<td>SVM</td>
<td>modified mathematical molecule for integration in the vertical direction with (\text{MYE}) or ((\text{MYE}+1)) elements</td>
</tr>
<tr>
<td>P</td>
<td>present overburden effective stress at (\text{MYE}) points</td>
</tr>
<tr>
<td>PC</td>
<td>preconsolidation stresses at (\text{MYE}) points</td>
</tr>
<tr>
<td>PLOG</td>
<td>natural logarithm of the ratio between the pre-consolidation and the overburden stresses</td>
</tr>
</tbody>
</table>
Method

The computations are performed first for the upper layer; then, the displacements of the lower layer are evaluated by making the same computations with redefined parameters. A lower layer must be considered only if \( KKK < MYE \).

2.24 Subroutine STAB

Purpose

To search automatically for the minimum factor of safety.

Usage

CALL STAB(XC,YC,R,XINP,YINP,MINP,MX,MYE,SU,FX,D,DM,YY)

Description of Parameters

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>XC</td>
<td>X-coordinate of the center of the circular slip circle</td>
</tr>
<tr>
<td>YC</td>
<td>corresponding Y-coordinate</td>
</tr>
<tr>
<td>R</td>
<td>radius of the circular slip surface</td>
</tr>
<tr>
<td>XINP</td>
<td>X-coordinates of the points defining the embankment contour</td>
</tr>
<tr>
<td>YINP</td>
<td>corresponding Y-coordinates</td>
</tr>
<tr>
<td>MINP</td>
<td>number of points (XINP,YINP)</td>
</tr>
<tr>
<td>MX</td>
<td>number of equally spaced grid points in horizontal direction</td>
</tr>
</tbody>
</table>
MYE  - number of equally spaced grid points in the vertical direction
SU   - undrained shear strengths at (MX*MYE) grid points
FX   - resulting factor of safety
D    - maximum step size to be used in the search procedure
DM   - minimum step size to be used in the search procedure
YY   - minimum permissible value for YC

Method and Reference

The programmed method embraces two tactical manoeuvres, the "exploratory move" and the "pattern move". Starting from the input base point (XC,YC), an exploratory move is made by varying first XC and then YC. If this move is successful, a pattern move is performed, followed again by a pattern move, if it was successful, and by an exploratory move, if it was not successful. This procedure is repeated until the minimum has been detected, whereafter the step size, by which XC and YC are varied, is decreased. When the minimum factor of safety is found by use of the smallest step size, DM, it is checked to determined whether the corresponding slip circle outcrops in front of the toe of the embankment. If it does not, an additional search is started and the smaller of the obtained minimum factors of safety is returned together with the coordinates and the radius of the corresponding arc.
Subroutines Required

VARYR(YC,XC,R,XINP,YINP,MINP,MX,MYE,SU,FS,DMIN,YY)

Remarks

The input data XC and YC are destroyed and replaced by the coordinates of the arc which gives the minimum factor of safety.

The step size is decreased in the subroutine by dividing by 2; it is, thus, possible that the smallest step size used is less than the input value DM.

2.25 Subroutine VARYR

Purpose

To vary the radii of trial arcs which have the same center coordinates and to compute the associated factors of safety.

Usage

CALL VARYR(YC,XC,R,XINP,YINP,MINP,MX,MYE,SU,FS,DMIN,YY)

Block Names

SAPOD/IOUTP*,W*,H,GLOAD*,CLOAD*,NARC*,NRAD;
* parameters marked by an asterisk are not needed in this subroutine

Description of Parameters

YC - Y-coordinate of the center of the circular slip surfaces
Method and References

After determination of the maximum and minimum possible radii, RMAX and RMIN, respectively, the factors of safety are computed for NRAD radii \( R = R_{MAX} - I \times \frac{(R_{MAX} - R_{MIN})}{(NRAD - 1)} \). The minimum value of the so-obtained NRAD factors of safety is returned to the calling program.

Subroutines Required

DETFS(XC,YC,R,XINP,YINP,MINP,MX,MYE,SU,FS)
3. **Input Data**

The main program is written in an interactive manner so that the user will be able to input the following data.

1. Name of the input file (length limited to 50 spaces)

2. Name of the output file (length limited to 50 spaces)

3. Number of symbols to be used in the output graph after execution

4. The characters that are used in the output graph after execution

On execution of the program the following messages will appear on the screen one by one and the user should input the corresponding data.

1. Specify the name of the input file

2. Specify the name of the output file

3. Specify the number of symbols to be used (usually 4)

4. Specify the characters - blank, star, grid, symb(I) I=1, mmm

After specifying these data, the program will use the data on the specified input file and write the results on the specified output file.
Proposed characters are:

Blank - a blank space
Star - *
Grid - I
Symb(1) = U - for avg. degree of consolidation as a percentage of reference load
Symb(2) = C - consolidation settlement as a percentage of that of reference load
Symb(3) = O - immediate settlement as a percent of that of reference load
Symb(4) = T - total settlement as a percent of that of reference load

(See sample problems for the proper use of these)

This section illustrates the sequence of input data in the main program. A free format style is used. These input data must be given in an input file (the name of the input file is specified by the user).

(a) Input data corresponding to the mesh-generation for the numerical solution, in the compressible layer and type of analysis required.

1. MYE,MHE,ISP,HF,POR,IPOR

MYE - number of equivally spaced points in the vertical direction, including the surface
and the bottom of the compressible layer in the finite difference mesh in the compressible soil \( \text{MYE} < 12 \)

**MHE** - number of points in the horizontal direction (mesh points) in the compressible soil (MHE < 40)

**ISP** - identifier where
- ISP=0 - settlements, the process of consolidation and the stability are analyzed
- ISP=1 - settlements and the process of consolidation are analyzed. Times of load application are required as input parameters

**HF** - identifier where
- HF=0.0 - horizontal flow is neglected in the process of consolidation
- HF=1.0 - horizontal flow is not neglected in the process of consolidation

**POR** - ratio horizontal drainage distance divided by \((X(T(IEND))*W)\) in the case of ISP=1, and Set POR=1.0 if this is not known

**IPOR** - identifier where
- IPOR=1 - The user provides the value of POR
- IPOR=0 - the user provides POR=1.0 the program will calculate the value of POR

2. **JND** (add this card only if ISP=0)

**JND** - number of points in the horizontal direction for which output are required

\( \text{JND} < 10 \)

3. **(JSP(K),K=1,JND)** (add this card only if ISP=0)

**JSP(K)** - indices of the JND points for which output is required. \( K=1, \text{JND} < 10 \); the output is for points \( X(E(JSP(K))) \), where \( X(E) \) are MX equally spaced coordinates between
and including the limits AX(1)*W and AX(INI)*W; for example, specification of JSP(1)=1, JSP(JND)=MX causes the output of information at the limits W*AX(1) and W*AX(INI) respectively.

4. LND

LND  - number of weeks to be plotted on the
time axis of the graphical output

5. MX,NI (add this card only if ISP=0)

MX  - number of equidistant points in the X-direction
     between limits AX(I) and AX(INI)
     MX < 51 User may chose MX as equal to
     MHE

NI  - number of interval limits AX(I); NI < 5

Notes:

AX(INI)*W is the last point (which is considered in the
analysis) in the horizontal direction from the CL. AX(1)=0.0 is
the centerline of the embankment.

This horizontal distance between AX(1) and AX(INI) is divided
into NI subdivisions. Each subdivision is further divided into
MXT points. User may typically use 3-4 subdivisions for each
interval between AX(I)*W and AX(I+1)*W. (Ref. Krizek and Krug-
man, 1972 for details.)

6. (AX(I)=1,NI) (add this card only if ISP=0)

AX(I)  - sub-interval limits as decimal fractions of
     reference value W (select values such
     that a smooth curve along the pore pressure
     vs. AX(I)*W will give the expected
     shape of the pore pressure distribution)
     (Hint - let contour points of the embankment
     be some of the AX(I)*W)
7. (MXT(I), I=1, NI-1) (add this card only if ISP=0)

MXT(I) - number of unequivalently spaced points XT between the consecutive limits AX(I) and AX(I+1). I=1, NI-1. Maximum value of MXT(I) < 10. Sum of all MXT(I) < 20

8. IEND (add this card only if ISP=1)

IEND - number of points in the horizontal direction for which output are required

9. (XT(I) I=1, IEND) (add this card only if ISP=1)

XT(I) - X-coordinates of the points at which the settlements and the consolidation behavior are determined. Output are printed for points at distance XT(I)*W

(b) This section gives data corresponding to the compressible layer under the embankment.

10. H, GLOAD, CLOAD, W, YWM, TGPHI

H - thickness of the compressible layer dimension (ft)
If H=0 the program is terminated

GLOAD - unit weight of the embankment soil (pcf)

CLOAD - undrained strength of the embankment soil (psf)

W - reference value in the horizontal direction (ft)
YWM - thickness of a drainage blanket placed on the surface of the compressible soil layer (ft)

TGPHI - tangent of the angle of internal friction of the drainage blanket, if there is one

11. IBCV, LAYER

IBCV - identifier where
- IBCV=1 - impeded drainage at Y=H
- 2 - free drainage at Y=H
- 3 - no drainage at Y=H

LAYER - number which indicates the location of a layer interface; e.g. LAYER=KK means that the layer interface is located at a depth below ground surface which is equal to
\[ Y = \frac{H \times (KK-1)}{MYE-1} \]
If only one type of soil is to be considered SET LAYER=0
4 LE' LAYER' LE'(MYE-3)

12. HI, RKV (add this card only if IBCV=1)

HI - thickness of impedance layer

RKV - ratio of vertical permeabilities
\[ \frac{K(\text{drainage soil})}{K(\text{impedance layer})} \]

13. RK, RC, REO, RAV, RCC, ROCL (add this card only if LAYER > 3)

RK - ratio of vertical permeabilities
K(lower soil) 
K(upper soil)

RC  - ratio of (vertical) coeff. of consolidation $c_v$
$c_v$(lower soil) 
$c_v$(upper soil)

ROCL  - ratio of recompression index of lower soil to that of upper soil

REO  - ratio of initial void ratios
$e_o$(lower soil) 
$e_o$(upper soil)

RCC  - ratio of virgin compression index $c_c$
$c_c$(lower soil) 
$c_c$(upper soil)

RAV  - ratio of coefficient of compressibility $a_v$
$a_v$(lower soil) 
$a_v$(upper soil)

14. IVAR, IAV, ICV

All these are identifiers where

IVAR=0  - use constant coeff. of consolidation

IVAR=1  - use variable coeff. of consolidation which are obtained either by interpolation between CHIN(I) and CVIN(I) or by varying the coeff. of compressibility and/or the coefficient of permeability (i.e., if ICV > 0)

IAV=0  - use a constant coeff. of compressibility in the settlement computations

IAV=1  - use the compression indices in the settlement computations

ICV  - number of data pairs [PCV(I), CVIN(I)] and [PCH(I), CHIN(I)] through which
Lagrangean/Linear interpolation polynomials are passed
0 < ICV < 0

15. EO, A

EO - initial void ratio of the upper soil
A - Skempton's pore pressure coefficient (A)

16. AVO (add this card only if IAV=0)

AVO - constant coeff. of compressibility to be used in the settlement computations; in the case of two layers AVO applies to the upper layer (ft²/lb)

17. CC, ROC, GAMMA (add this card only if IAV=1)

CC - virgin compression index (in the case of two layers this applies to the upper layer)

ROC - ratio of recompression index to the virgin compression index (in the case of two layers - upper layer)

GAMMA - effective unit weight of the subsoil, constant over the thickness of the compressible layer
If GAMMA=0, input of MYE effective overburden stress at equivalently spaced depths must be input (pcf)

18. P(I), PC(I) (add this card only if GAMMA=0) (total of MYE cards)
P(I) - present overburden effective stress at MYE equally spaced depths, including the surface MYE < 12 (psf)

PC(I) - preconsolidation stresses at MYE equally spaced depths, including surface, MYE < 12 (psf)

19. CV, CH (add this card only if IVAR=0 and ICV=0)

CV - constant coeff. of consolidation in the vertical direction (in the case of two layer-upper layer) (ft^2/day)

CH - constant coeff. of consolidation in the horizontal direction (in the case of two layers-upper layer) (ft^2/day)

20. PCV(I),CVIN(I),PHC(I),CHIN(I) (add this card only if IVAR=1 and ICV>0) (ICV number of cards)

PCV(I) - effective stresses at which the vertical coeff. of consolidation CVIN(I) are defined ICV < 10 (psf)

PCH(I) - effective stresses at which the horizontal coeff. of consolidation CHIN(I) are defined (psf) ICV < 10

CVIN(I) - variable coefficients of consolidation in the vertical direction (interpolation is done in subroutine coef) (ft^2/day) ICV < 10

CHIN(I) - variable coeff. of consolidation in the horizontal direction (interpolation is done in subroutine coef) (ft^2/day) (ICV < 10)

21. KVO,KHO (add this card only if IVAR=1 and ICV=0)
KVO  - initial coeff. of permeability in the horizontal direction
KHO  - initial coeff. of permeability in the vertical direction
(ft/day)

22. ISAT,IK (add this card only if IVAR=1 and ICV > 0)

These are identifiers where

ISAT=0  - complete saturation
       =1  - partial saturation; requires
            Skempton's pore pressure
            parameter B < 1.0

IK=0  - constant coefficient of permeability
       =1  - horizontal and vertical permeabilities
            are variables; void ratio vs. the
            logarithm of the coeff. of
            permeability is a straight line

23. S,PU,HC,B (add this card only if IVAR=1, ICV > 0 and ISAT=1)

S  - degree of saturation to be input as a
decimal fraction < 1.0

PU  - initial pore gas pressure, if PU is
     not defined during input, it is set
equal to the sum of the atmosphere
     pressure plus one half the thickness
     of the compressible layer times
     the unit weight of water (psf)

HC  - Henry's constant of gas solubility,
     HC=0.020 for atmospheric air,
     HC=0.029 for methane, HC=2.84
     for hydrogen sulfide (at 68°F)

B  - Skempton's pore pressure coefficient (B)

24. SKV,SKH (add this card only if IVAR=1, ICV>0, IK=1)
SKV - slope of the void ratio vs. logarithm of vertical coeff. of permeability

SKH - slope of the void ratio vs. logarithm of horizontal coeff. of permeability

25. NC (add this card only if ISP=0)

NC - number of initial undrained shear strengths, CO(I), and (c/p)-ratios, CP(I);
NC < MYE < 12

26. Y UA(I), UB(I) (add this card only if ISP=0) (total of NC cards)

Y - vertical distance below the ground surface at which the initial shear strengths are given (positive downwards) (ft)

UA(I) - initial undrained shear strength CO(I) at depth Y (psf)

UB(I) - (c/p)-ratio at depth Y

* At this point the program will write the data on the OUTPUTFILE.

(c) Following data corresponds to the REFERENCE LOAD.

27. MINP, NS

MINP - the number of points where the coordinates of the embankment will be given. This defines the contour of the embankment (MINP < 20)

NS - number of load strips to approximate the actual embankment load (NS < 20)
28. XINP(I), YINP(I) (total of MINP cards)

XINP(I) - X-coordinates of the points defining the embankment contour (ft) (MINP < 20)

YINP(I) - Y-coordinates of the points defining the embankment contour (ft) (MINP < 20)

(d) Following data corresponds to the load application (i.e. each step of load to the embankment).

29. NL,(IDEN(I),I=1,NL)

NL - number of load strips (NL < 6)

IDEN(I) - identifier corresponding to the Ith load strip; where

IDEN(I) < 0 - the excess pore pressure due to the first load step are equal to the input residual pore pressure

IDEN(I) = 0 - the excess pore water pressure due to the Ith load are computed by means of subroutine PORE

IDEN(I) = 1 - the excess pore pressure due to the Ith load are set equal to those computed for the reference load. Note, that this requires that Skempton's coefficients A and B are identical in both cases

IDEN(I) < 0 - allows the check of an existing installation for which the excess pore pressures just after load application are known from field measurements

30. (TL(I),I=1,NL) (add this card only if ISP=1)
TL(I) - times of load application (i.e. each step of load) in case where ISP=1 NL < 6 (days)

31. FSI, SPEC(1), SPECU(1), TA, DMAX, DMIN, XC, YC, ZZ (add this card only if ISP=0)

FSI - factor of safety required at the time of application of the first load

SPECS(1) - specified fraction of the consolidation settlement due to the reference load. This settlement must have occurred before a new load is applied.

SPECU(1) - when the non-dissipated average pore pressures become less than 5% of the total average pore pressure just after the application of the last load at IEND*SPECU points XT the subsequent loads are disregarded. SPEC is a decimal fraction. IEND is the total number of points in the X-direction created by the program (see Sec. 2.1)

TA - available construction time. This is the time at which the final load must have been applied (days)

DMAX - maximum step size to be used in the variation of XC and YC in the search procedure for the minimum factor of safety (ft)

DMIN - minimum step size to be used in the variation of XC and YC in the search procedure for the minimum factor of safety (ft)

XC - X-coordinate of the center of the first trial arc, If XC=0 is input, the program selects a starting value (ft)

YC - Y-coordinate of the center of the first trial arc, If YC=0 is input, the program selects a starting value (ft)
(Note - positive upward)
32. NARC,NRAD (add this card only if ISP=0)

NARC - one-half the number of subarcs within the subsoil to be used in subroutine DETFS; NARC ≥ 1

NRAD - number of trial arcs to be used with each trial center (XC,YC) in the stability analysis; NRAD ≥ 1

33. MINP,NS,IAB (gives data corresponding to the first load)

MINP - same as defined earlier - corresponding to the first load

NS - same as defined earlier - corresponding to the first load

IAB - identifier where
  IAB=0 - Skempton's pore pressure coefficients A and B as defined for the last load are also used to compute the pore pressures due to the load addition
  IAB=1 - redefine A and B (i.e. assign new values for A and B)

34. XINP(I),YINP(I) (total of MINP cards)

XINP(I) - as defined earlier - corresponding to the first load

YINP(I) - as defined earlier - corresponding to the first load

35. A,B (add this card only if IBA=1)

A,B - corresponding Skempton's pore pressure parameters (new)
At this point of the program, if satisfactory FS is not reached, within the available construction time, for the first load the program is terminated (i.e., no loads can be added to the existing embankment). Note - this is for ISP=0

(e) The following data refers to the residual pore pressures under the embankment.

36. IRP

IRP - identifier where
IRP=0 - no residual pore pressures are input
IRP=1 - residual pore pressure at points (W'XT, H'YE) are input columnwise
IRP=2 - residual pore pressure at arbitrary points are input

37. (UC(I),I=1,ISUM) (add this card only if IRP=1)

UC(I) - residual pore pressures under the embankment at points (W'codtXT, H'YE) are input columnwise (psf)

38. X,Y,UA(I),COUNT (add this card only if IRP=2) (number of cards depends on number of arbitrary pore pressures to be given)

X - the X-coordinate at which residual pore pressure UA(I) is specified

Y - the Y-coordinate at which residual pore pressure UA(I) is specified

COUNT - identifier where
COUNT=0.0 - in all the cards except the last card
COUNT=1.0 - the last data card on residual pore pressure

At this point of the program, the internal file unit 1 will be rewinded to initiate the recording of the output data. The program calculates the required parameters corresponding to the first load.
Steps in this section should specify second and following loads for a total of (NL-1) lifts with proper values corresponding to each step of load, i.e. total of NL load applications NL < 6 are allowed.

(f) The following data corresponds to the second and following loads. [This data should cover (NL-1) loading steps.]

39. MINP,NS,IAB (gives data corresponding to the 2nd load step)

\[
\begin{align*}
\text{MINP} & \\
\text{NS} & \text{as defined earlier - corresponding to the 2nd load step} \\
\text{IAB} & 
\end{align*}
\]

40. XINP(I),YINP(I) (gives the contour of the embankment for the 2nd or following load steps) (total number of cards = MINP)

\[
\begin{align*}
\text{XINP(I)} & \quad \text{as defined earlier - corresponding to the} \\
\text{YINP(I)} & \quad \text{2nd or following load step}
\end{align*}
\]

41. A,B (add this card only if IAB=1)

\[
\begin{align*}
\text{A} & \quad \text{pore pressure parameters} \\
\text{B} &
\end{align*}
\]

42. FSI,SPECS(LL),SPECU(LL),TMIN,XC,YC,ZZ (add this card only if ISP=0)

\[
\begin{align*}
\text{FSI} & \quad \text{specified required factor of safety for the 2nd load step} \\
\text{SPECS(LL)} & \quad \text{specified fraction of the consolidation settlement due to reference load that must}
\end{align*}
\]
have occurred before the addition of the next load step

**SPECU(LL)** - if an average degree of consolidation of 95% due to the LIFT-TH load is obtained at SPECU(LIFT)*IEND points XT without a sufficient factor of safety for the present load (i.e., LLth load) the LIFTth load is taken to be the last load and NL is set at NL=LIFT (i.e., the present load will not be added to the embankment). SPECU is input as a decimal fraction (see Sec. 2.1)

**TMIN** - time which must have passed after a load application before the first stability is made to determine whether the next load can be applied. (This saves unnecessary computer time in calculating FS before sufficient pore pressure is dissipated.)

**XC, YC, ZZ** - as defined previously - corresponding to the LLth load

**Note:** The program can handle analysis of multiple embankments in a single run.

This is done by adding the card No. 10 at the end of the cards for the previous embankment but with replacing $H = 99$.

This makes the program to goto the beginning of cards. For second or following embankments data cards should be repeated from 1-42 as in the case of first embankment.

To terminate the program at the end of analysis of nth embankment, simply add the card #10 with $H = 0$ at the end of data cards for the nth embankment.

4. **Sample Problems**

Two sample problems have been prepared to show some of the features of the computer programs. In the first problem, only a settlement analysis for a specified load history is required, whereas the second problem simulates actual design conditions.
using soil data and cross-section from an unpublished report by the STATE OF ILLINOIS, DIVISION OF HIGHWAYS (1967).

4.1 Sample Problem for Settlement Analysis (Sample Problem #1)

In this first problem the load, which includes a surcharge of 5 feet, is applied at time TL(1)=60 days, and the surcharge is removed at time TL(2)=160 days. The geometry is given in Figure 4.1, and the soil parameters, which are assumed to vary during consolidation, are compiled in Table 4.1 and in Figure 4.2. Output of pore water pressures is required at specified points under the embankment. The input sequence for the soil parameters, the geometry, and the load characteristics follows the list of data cards given in Section 3 for program "Modified SAND" with ISP=1, and the contents of the data cards are listed in Table 4.2 in Appendix A.

The computer output has been abridged, where it was repetitive in nature, and is reproduced in Figure 4.3 in Appendix A.

4.2 Sample Design Problem (Sample Problem #2)

The geometry for this problem is depicted in Figure 4.4 together with a summary of the soil conditions deduced from the boring log shown in Figure 4.5 and the consolidation test data of Figure 4.6. To account for the smaller initial void ratio and larger coefficients of consolidation near the ground surface, it was decided to introduce a layer interface at a depth of (H-H')=5.2 feet, corresponding to LAYER=3.
The design must satisfy the following requirements: (1) No settlements due to primary consolidation must occur after surcharge removal; in addition, some settlements due to secondary compression should be eliminated; (2) the construction time is not to exceed 12 months; and (3) the factor of safety against instability of the embankment-subsoil system must be equal to or greater than 1.15 during construction and 1.25 under the final load.

The input sequence for the soil parameters, the geometry, and the load characteristics follows the input data in Section 3 for program Modified SAND with ISP=0, and the contents of the data cards are listed in Table 4.3 (Appendix A). The final output includes average degrees of consolidation for a point at the center of the embankment and another point close to the embankment toe, and is reproduced in Figure 4.7 (Appendix A).

4.3 Summary and Conclusions

The objectives of this study were (a) to elucidate the practical and theoretical bases for using the controlled rate of construction technique to design a highway embankment underlain by soft ground, and (b) to synthesize presently available procedures in a comprehensive computer program in which special attention is given to the horizontal and vertical drainage, without sand drain installations. An existing program SAND which considers sand drains has been modified for this purpose.
To facilitate the mathematical treatment, the overall problem was conveniently divided into four parts, which deal with (a) the initial increase in excess pore water pressures caused by an increase of the vertical load on the surface of the compressible layer, (b) the process whereby these pore water pressures are dissipated with time, (c) the associated settlements, and (d) the stability of the embankment-foundation system.

Based on the effective stress principle, the stress increases associated with primary consolidation are taken to be equal to the dissipated pore water pressures. The latter are computed by means of Skempton's pore pressure coefficients A and B and a solution for the total stresses due to a symmetric vertical load acting on a linearly elastic layer of finite thickness, which, in turn, is underlain by a rough rigid substratum. The dissipation of excess pore water pressures is evaluated by use of a consolidation theory which accounts for horizontal and vertical drainage conditions, anisotropic permeability, time-dependent variations of the soil parameters, and partial saturation. As a result of the increases in effective stresses due to the dissipation of pore water pressures, the strength of the subsoil increases, and this is considered in a stability analysis in terms of total stresses by use of the $c/\bar{p}$-ratio.
It is economically possible to establish a number of design charts, which include (a) excess pore pressure distribution curves, (b) consolidation-time curves, (c) stability charts, (d) graphs of maximum embankment height versus thickness of the compressible layer, and (e) relationships for equivalent uniform strength after complete consolidation versus thickness of the subsoil. However, this is beyond the scope of this report.

Since the computational technique used in the "Modified Sand" remains the same as in the original program sand the following remarks are valid.

A. With regard to the computation of the initial excess pore water pressure distribution, the following conclusions can be drawn:

1. The form of the stress equations requires the numerical integration of oscillating integrands, and convergence of the extended Simpson's rule or Filon's formulae with interval halving depends on the geometry of the problem. Poorest convergence was obtained in cases of heavily oscillating integrands when the ratio of the load width to the thickness of the compressible layer was large.

2. When the pore pressure coefficient B is held constant and equal to unity, the influence of the pore pressure coefficient A increases as the thickness of the compressible layer increases, and the average pore water pressures are larger and extend farther in the horizontal direction when A is larger.
3. As the compressible layer becomes thinner relative to the load width, closer agreement is obtained between the applied vertical load and the resulting average pore pressure distribution.

4. The influence of shear stresses causes some concentration of average pore water pressures near the edges of the load.

B. With regard to the computation of primary consolidation settlements, direct proportionality between the average degree of consolidation and the resulting settlement will occur only when constant coefficients of consolidation and a constant coefficient of compressibility are used.

C. With regard to the stability analyses, the following conclusions can be drawn from a critical comparison of the charts in the report (Krizek and Krugman, 1972):

1. Depending on the geometry of the embankment and the soil parameters of the embankment and the subsoil, the assumption of a circular slip surface will give reliable factors of safety only for sufficiently large subsoil thicknesses.

2. The stabilizing influence of flattening the embankment slope decreases as the thickness of the subsoil increases.
3. The slip circle resulting in a minimum factor of safety generally tends to penetrate the soft subsoil as deep as possible.

4. The factor of safety is not proportional to the height of the embankment, but, given identical soil parameters, it depends on the ratio of the embankment height and the thickness of the compressible layer.

Reference

Fig. 4.1 Contour of the Embankment Configuration for the Sample Problem 1
Figure 4.2 Soil Characteristics of a Sample from Depth of 8 ft below Ground
Table 4.1 Soil Data for the First Sample Problem

<table>
<thead>
<tr>
<th>Soil Parameter</th>
<th>Embankment</th>
<th>Subsoil</th>
</tr>
</thead>
<tbody>
<tr>
<td>Unit weight (pcf)</td>
<td>125</td>
<td>58.8</td>
</tr>
</tbody>
</table>
| Initial coefficient of permeability (feet/day)      |            | $K_{HO} = 6.26 \times 10^{-3}$  
            |            | $K_{VO} = 3.08 \times 10^{-3}$  |
| Slope of the void ratio versus log coefficient of permeability curve |            | SKH = 0.35                 
            |            | SKV = 0.40                 |
| Initial void ratio, $e_0$                           |            | 1.175                    |
| Compression index, $C_c$                            |            | 1.18                     |
| Skempton's pore pressure coefficients               |            | $A = 0.5$                 
            |            | $B = 0.95$                |
| Degree of saturation, $S$                           |            | $S = 0.98$                |
| Henry's coefficient of gas solubility, $HC$         |            | 0.02                     |
Table 4.2 Data Cards for Sample Problem 1

11, 40, 10, 1, 1.000, 1.00, 0
40
10
0.0, 1, 2, 3, 4, 5, 7, 9, 1.5, 1.600
16.0, 125.0, 1000.0, 100.0, 2.0, 0.0
3, 0
1, 1, 0
1.175, 0.500
1.18, 1.0, 58.80
0.00308, 0.00625
1, 1
0.98, 0.0, 0.2, 0.95
0.4, 0.35
3, 5
0.0, 5.0
40.0, 5.0
70.0, 0.0
2, 0, 1
60.0, 160.0
4, 10, 0
0.0, 10.
30.0, 10.0
40.0, 5.0
70.0, 0.0
2
35.0, 4.0, 75.0, 0.0
35.0, 15.0, 75.0, 0.0
105.0, 8.5, 25.0, 1.0
3, 5, 0
0.0, 5.0
40.0, 5.0
70.0, 0.0
0.0, 0.0, 0.0, 0.0, 0.0, 0.0, 0.0
Abridged output for Sample Problem 1

The pore water pressures are computed at

<table>
<thead>
<tr>
<th>YE/H</th>
<th>0.000</th>
<th>0.100</th>
<th>0.200</th>
<th>0.300</th>
<th>0.400</th>
</tr>
</thead>
<tbody>
<tr>
<td>YE/H</td>
<td>0.500</td>
<td>0.600</td>
<td>0.700</td>
<td>0.800</td>
<td>0.900</td>
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<tr>
<td>YE/H</td>
<td>1.000</td>
<td></td>
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<td></td>
<td></td>
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<td>XT/W</td>
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<td>0.100</td>
<td>0.200</td>
<td>0.300</td>
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<td>XT/W</td>
<td>0.500</td>
<td>0.700</td>
<td>0.900</td>
<td>1.500</td>
<td>1.600</td>
</tr>
</tbody>
</table>

The subsoil is described by the following parameters which are given for the upper layer in case of stratification:

Total thickness \( H = 16.000 \) feet

Reference for X-coord \( W = 100.000 \) feet

Skempton pore pressure coefficients are

\( A = 0.50 \) and \( B = 0.95 \)

Degree of saturation is \( S = 0.980 \)

Henry's constant of gas solubility \( HC = 0.200 \)

Initial pore gas pressure is \( PU = 0.2616E+04 \) PSF

Initial void ratio = 1.175

The compression index is = \( 0.1180E+01 \)

Recompression index/CC \( ROC = 1.000 \)

Initial effective \( P \) and precompression stresses \( PC \) as used in the computations

\( Y \) in FT \( P \) in PSF \( PC \) in PSF

<table>
<thead>
<tr>
<th>( Y ) in FT</th>
<th>0.000</th>
<th>1.600</th>
<th>3.200</th>
<th>4.800</th>
<th>6.400</th>
<th>8.000</th>
<th>9.600</th>
<th>11.200</th>
<th>12.800</th>
<th>14.400</th>
<th>16.000</th>
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</thead>
<tbody>
<tr>
<td>( P ) in PSF</td>
<td>94.08</td>
<td>94.08</td>
<td>188.16</td>
<td>282.24</td>
<td>376.32</td>
<td>470.40</td>
<td>564.48</td>
<td>658.56</td>
<td>752.64</td>
<td>846.72</td>
<td>940.80</td>
</tr>
<tr>
<td>( PC ) in PSF</td>
<td>94.08</td>
<td>94.08</td>
<td>188.16</td>
<td>282.24</td>
<td>376.32</td>
<td>470.40</td>
<td>564.48</td>
<td>658.56</td>
<td>752.64</td>
<td>846.72</td>
<td>940.80</td>
</tr>
</tbody>
</table>

Note: \( P(1) \) and \( PC(1) \) may have been changed as compared to input values to avoid overflow.

The initial permeabilities are in vertical dirn. \( KVO = 0.3080E-02 \) FT/DAY

Horizontal dirn \( KHO = 0.6250E-02 \) FT/DAY

The slopes of the \( E \) vs \( \log(K) \) curves are

In vertical dirn, \( SKV = 0.400 \)

In hori. dirn, \( SKH = 0.350 \)

The drainage conditions are
NO DRAINAGE AT THE BOTTOM

REFERENCE LOAD

SKEMPTON PORE PRESSURE COEFFICIENTS ARE
A = 0.50 AND B = 0.95

THE LOAD CHARACTERISTICS ARE GIVEN BY
THE UNIT WEIGHT GLOAD = 125.00 PCF
THE COHESION, CLOAD = 1000.00 PSF
THICKNESS OF THE DRAINAGE BLANKET YWM = 2.00 FT
THE TANGENT OF THE ANGLE OF INTERNAL
FRICITION Tgphi = 0.0000

MINP = 3 COOR POINTS XINP/YINP

THE ACTUAL LOAD IS APPROXIMATED BY 5 LOADS
OF EQ INTENSITY WHICH EXTEND FROM X=0 TO ALPHA(I)
IF ALPHA(I) IS NEGATIVE, THIS LOAD IS SUBTRACTED

ALPHA( 1) = 67.000 FEET
ALPHA( 2) = 61.000 FEET
ALPHA( 3) = 55.000 FEET
ALPHA( 4) = 49.000 FEET
ALPHA( 5) = 43.000 FEET

THE AVERAGE PORE PRESSURES, UAVER(I)
THE CONSOLIDATION SETTLEMENTS, SETRC(I) AND
THE TOTAL SETTLEMENTS, SETR(T), AT XT(I) ARE

<table>
<thead>
<tr>
<th>XT</th>
<th>FEET</th>
<th>UAVER (PSF)</th>
<th>SETRC FT.</th>
<th>SETR FT</th>
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<tr>
<td>0.00</td>
<td>585.99</td>
<td>3.781</td>
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<td>10.00</td>
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<td>3.777</td>
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<td>3.876</td>
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<td>30.00</td>
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<td>70.00</td>
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<td>1.180</td>
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<td>90.00</td>
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<td>0.539</td>
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<tr>
<td>160.00</td>
<td>1.52</td>
<td>0.023</td>
<td>0.024</td>
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</table>

THE NUMBER OF LIFTS IS NL = 2

SINCE ISP = 1 TIMES OF LOAD APPLICATION
ARE INPUT TO BE

TL( 1) = 60. DAYS
TL( 2) = 160. DAYS

RESIDUAL PORE PRESSURES ARE IN PUT AS

<table>
<thead>
<tr>
<th>X (FEET)</th>
<th>Y (FEET)</th>
<th>UC (PSF)</th>
</tr>
</thead>
<tbody>
<tr>
<td>35.000</td>
<td>4.000</td>
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<td>75.000</td>
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<tr>
<td>105.000</td>
<td>8.500</td>
<td>25.000</td>
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</table>

LOAD NO 1 APPLIED AT TL = 60. DAYS

********************************************************************
SKEMPTON PORE PRESSURE COEFFICIENTS ARE 
A = 0.50 AND B = 0.95

THE LOAD CHARACTERISTICS ARE GIVEN BY
THE UNIT WEIGHT GLOAD = 125.00 PCF
THE COHESION, CLOAD = 1000.00 PSF
THICKNESS OF THE DRAINAGE BLANKET YWM = 2.00 FT
THE TANGENT OF THE ANGLE OF INTERNAL
FRICTION TGPHI = 0.0000
MINP = 4 COOR POINTS XINP/YINP
0.00 FEET  10.00 FEET
30.00 FEET  10.00 FEET
40.00 FEET  5.00 FEET
70.00 FEET  0.00 FEET

THE ACTUAL LOAD IS APPROXIMATED BY 10 LOADS
OF EQ INTENSITY WHICH EXTEND FROM X=0 TO ALPHA(I)
IF ALPHA(I) IS NEGATIVE, THIS LOAD IS SUBTRACTED

ALPHA( 1) = 67.000 FEET
ALPHA( 2) = 61.000 FEET
ALPHA( 3) = 55.000 FEET
ALPHA( 4) = 49.000 FEET
ALPHA( 5) = 43.000 FEET
ALPHA( 6) = 39.000 FEET
ALPHA( 7) = 37.000 FEET
ALPHA( 8) = 35.000 FEET
ALPHA( 9) = 33.000 FEET
ALPHA(10) = 31.000 FEET

T= 0. DAYS X/W 0.000 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW

0.000
1217.070
1220.074
1223.127
1226.202
1229.288
1232.371
1235.432
1238.484
1241.515
1244.547

T= 0. DAYS X/W 0.100 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW

0.000
1203.986
1207.619
1211.390
1215.356
1219.585
1224.196
1229.339
1235.257
1242.284
1250.778

\[ T = 0.000 \text{ DAYS X/W} \quad 0.200 \text{ PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW} \]

0.000
1184.936
1185.586
1186.735
1188.883
1192.537
1198.191
1206.311
1217.356
1231.762
1249.895

\[ T = 0.000 \text{ DAYS X/W} \quad 0.300 \text{ PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW} \]

0.000
1137.844
1106.615
1088.493
1078.696
1075.409
1077.865
1085.907
1099.824
1120.355
1148.747

\[ T = 0.000 \text{ DAYS X/W} \quad 0.400 \text{ PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW} \]
<table>
<thead>
<tr>
<th>Days X/W</th>
<th>0.500 Pore Pressures in PSF due to Vert + Hori Flow</th>
</tr>
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<table>
<thead>
<tr>
<th>Days X/W</th>
<th>0.700 Pore Pressures in PSF due to Vert + Hori Flow</th>
</tr>
</thead>
<tbody>
<tr>
<td>0.000</td>
<td>0.000</td>
</tr>
<tr>
<td></td>
<td>184.606</td>
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<td>198.226</td>
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<tr>
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<td>205.020</td>
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<td>207.824</td>
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<td>206.043</td>
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<td></td>
<td>202.389</td>
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<tr>
<td></td>
<td>197.170</td>
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<td>190.481</td>
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<td>182.376</td>
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</tbody>
</table>
T = 0. DAYS X/W
0.900 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW

0.000
99.416
96.627
93.800
90.903
87.904
84.783
81.525
78.128
74.598
70.955

T = 0. DAYS X/W
1.500 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW

0.000
-3.144
-3.296
-3.459
-3.630
-3.810
-3.996
-4.188
-4.386
-4.588
-4.793

T = 0. DAYS X/W
1.600 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW

0.000
-11.767
-11.867
-11.969
-12.077
-12.190
-12.308
-12.430
-12.554
-12.681
-12.811
<table>
<thead>
<tr>
<th>T= 7. DAYS X/W</th>
<th>0.000 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW</th>
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<tbody>
<tr>
<td>0.000</td>
<td>1005.060</td>
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<td>1221.441</td>
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<td>1232.104</td>
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<td>1235.166</td>
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<td>1243.217</td>
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<table>
<thead>
<tr>
<th>T= 7. DAYS X/W</th>
<th>0.100 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW</th>
</tr>
</thead>
<tbody>
<tr>
<td>0.000</td>
<td>994.349</td>
</tr>
<tr>
<td></td>
<td>1186.681</td>
</tr>
<tr>
<td></td>
<td>1209.759</td>
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<td>1215.080</td>
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<td>1219.404</td>
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<table>
<thead>
<tr>
<th>T= 7. DAYS X/W</th>
<th>0.200 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW</th>
</tr>
</thead>
<tbody>
<tr>
<td>0.000</td>
<td>977.065</td>
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<tr>
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<td>1164.192</td>
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<td>1184.553</td>
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<td>1188.139</td>
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<td>1191.965</td>
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<td>1197.717</td>
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<td>1205.925</td>
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<td></td>
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<td></td>
<td>1230.826</td>
</tr>
<tr>
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<td>1242.611</td>
</tr>
</tbody>
</table>
$T = 7 \text{ DAYS } X/W \quad 0.300 \text{ PORE PRESSURES IN PSF DUE TO VERT + HORI FLO}$

0.000
927.833
1084.815
1089.476
1076.676
1073.297
1075.710
1083.785
1097.741
1117.368
1135.339

$T = 7 \text{ DAYS } X/W \quad 0.400 \text{ PORE PRESSURES IN PSF DUE TO VERT + HORI FLO}$

0.000
577.542
721.089
757.299
776.237
790.899
803.662
815.556
827.204
838.624
846.717

$T = 7 \text{ DAYS } X/W \quad 0.500 \text{ PORE PRESSURES IN PSF DUE TO VERT + HORI FLO}$

0.000
414.049
509.569
527.355
535.724
543.083
549.781
555.599
560.358
563.843
565.502
DAYS X/W 0.700 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW

0.000
150.603
194.164
205.344
208.848
209.181
207.338
203.703
198.499
192.169
187.244

DAYS X/W 0.900 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW

0.000
78.568
94.598
94.130
91.414
88.405
85.261
81.984
78.583
75.209
72.924

DAYS X/W 1.500 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW

0.000
-1.739
-2.227
-2.388
-2.511
-2.635
-2.764
-2.896
-3.032
-3.165
-3.253
$T = 7. \text{ DAYS X/W} \quad 1.600 \text{ PORE PRESSURES IN PSF DUE TO VERT + HORI FLO}$

<table>
<thead>
<tr>
<th>Time (Days)</th>
<th>Pore Pressure (PSF)</th>
</tr>
</thead>
<tbody>
<tr>
<td>0.000</td>
<td>-9.327</td>
</tr>
<tr>
<td>0.100</td>
<td>-11.585</td>
</tr>
<tr>
<td>0.200</td>
<td>-11.964</td>
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<tr>
<td>0.300</td>
<td>-12.096</td>
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<td>0.400</td>
<td>-12.211</td>
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<tr>
<td>0.500</td>
<td>-12.329</td>
</tr>
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<td>0.600</td>
<td>-12.450</td>
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<tr>
<td>0.700</td>
<td>-12.574</td>
</tr>
<tr>
<td>0.800</td>
<td>-12.695</td>
</tr>
<tr>
<td>0.900</td>
<td>-12.776</td>
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</table>

$T = 49. \text{ DAYS X/W} \quad 0.000 \text{ PORE PRESSURES IN PSF DUE TO VERT + HORI FLO}$

<table>
<thead>
<tr>
<th>Time (Days)</th>
<th>Pore Pressure (PSF)</th>
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<td>623.692</td>
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<td>0.700</td>
<td>1236.544</td>
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<tr>
<td>0.800</td>
<td>1238.756</td>
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<tr>
<td>0.900</td>
<td>1239.663</td>
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</table>

$T = 49. \text{ DAYS X/W} \quad 0.100 \text{ PORE PRESSURES IN PSF DUE TO VERT + HORI FLO}$

<table>
<thead>
<tr>
<th>Time (Days)</th>
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<td>616.674</td>
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<tr>
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<td>992.046</td>
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<td>1149.208</td>
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<td>1200.274</td>
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<tr>
<td>0.400</td>
<td>1216.000</td>
</tr>
<tr>
<td>0.500</td>
<td>1223.013</td>
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<tr>
<td>0.600</td>
<td>1228.602</td>
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<td>1234.115</td>
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<tr>
<td>$T = 49 \text{ DAYS X/W}$</td>
<td>0.200 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW</td>
</tr>
<tr>
<td>-------------------------</td>
<td>--------------------------------------------------</td>
</tr>
<tr>
<td>0.000</td>
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<td>602.520</td>
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\( T = 49 \text{ DAYS X/W} \quad 0.500 \text{ PORE PRESSURES IN PSF DUE TO VERT + HORI FLO} \)

\[
\begin{array}{c}
0.000 \\
235.913 \\
401.698 \\
489.368 \\
528.088 \\
545.007 \\
554.048 \\
560.212 \\
564.747 \\
567.677 \\
568.711 \\
\end{array}
\]

\( T = 49 \text{ DAYS X/W} \quad 0.700 \text{ PORE PRESSURES IN PSF DUE TO VERT + HORI FLO} \)

\[
\begin{array}{c}
0.000 \\
86.603 \\
152.094 \\
190.724 \\
208.935 \\
215.348 \\
215.791 \\
213.319 \\
209.699 \\
206.487 \\
205.181 \\
\end{array}
\]

\( T = 49 \text{ DAYS X/W} \quad 0.900 \text{ PORE PRESSURES IN PSF DUE TO VERT + HORI FLO} \)

\[
\begin{array}{c}
0.000 \\
40.089 \\
69.237 \\
84.641 \\
89.974 \\
89.981 \\
87.780 \\
84.901 \\
82.121 \\
80.055 \\
79.277 \\
\end{array}
\]
T = 49. DAYS X/W  1.500 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW

0.000
-1.436
-2.565
-3.302
-3.748
-4.039
-4.265
-4.463
-4.633
-4.755
-4.800

T = 56. DAYS X/W  0.000 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW

0.000
594.797
975.207
1146.130
1205.399
1223.418
1229.753
1233.346
1236.194
1238.329
1239.177

T = 56. DAYS X/W  0.100 PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW

0.000
588.056
965.000
1135.251
1195.173
1214.436
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1228.264
1233.697
1238.223
1240.133
T = 56. DAYS X/W  0.200 PORE PRESSURES IN PSF DUE TO VERT + HORI FL

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T = 56. DAYS X/W  0.300 PORE PRESSURES IN PSF DUE TO VERT + HORI FL

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<th>Time (Days)</th>
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<td>1089.156</td>
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</tbody>
</table>

Reached end of file

THE CONSOL. PROCESS

********************
THE FOLLOWING INFORMATION IS OUTPUT
UAVE(x(1)), UAVE(x(2)),.......,
- AVERAGE DEGREES OF CONSOL. WITH RESPECT TO REF. LOAD
SETC(x(1)), SETC(x(2)),.......,
- CONSOL. SETTLEMENTS
SETI(x(1)), SETI(x(2)),.......,
- IMMEDIATE SETTLEMENTS
SETT(x(1)), SETT(x(2)),.......,
- CONSOLI. + IMMEDIATE SETTLEMENTS
LAST TWO LINE ARE ONLY OUTPUT
IF SOIL IS PARTIALLY SATURATED (B.NE.1.)
THE POINTS X(I) IN FEET ARE AS FOLLOWS

0.000 10.000 20.000 30.000 40.000 50.000 70.000 90.000 150.000 160.000

T = 60.
DAYS IS THE TIME OF LOAD APPLICATION

T = 60. DAYS

T = 67. DAYS

T = 74. DAYS

T = 130. DAYS
AVE DEGREE OF CONSOL. AND SETTLEMENT CURVES FOR POINT X= 0.00 FEET FROM CENTER LINE INTERVAL BETWEEN 2 GRID LINES.EQ.10% ABCISSA NUMBERS GIVE THE TIME IN WKS THE TOTAL SETTL DUE TO REFERENCE LOAD IS= 0.390E+01FT U -CURVE= AVE. DEGREE OF CONSOL. RELATIVE TO THE PORE PRESS DUE TO REF LOAD C -CURVE=CONSOL. SETTL. IN % OF REF. SETTLEMENT O -CURVE=IMMEDIATE SETTLEMENTS IN % OF THE REFERENCE SETTLEMENTS T -CURVE=TOTAL SETTLEMENTS IN % OF THE REFERENCE SETTLEMENT  

Fig. 4.3(a)
Fig. 4.3(b)

| 0 | 0.1 | 0.2 | 0.3 | 0.4 | 0.5 | 0.6 | 0.7 | 0.8 | 0.9 | 1.0 | 1.1 | 1.2 | 1.3 | 1.4 | 1.5 |
|---|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|

**I****I****I****I****I****I****I****I****I****I****I****I****I****I****

Ave degree of consol. and settlement curves for point x = 30.00 feet from center line.
Interval between 2 grid lines, eq. 10%.
Abscissa numbers give the time in wks.
The total settl. due to reference load is = 0.384E+01 ft.

U - Curve = Ave. degree of consol.
Relative to the pore press. due to ref. load.
C - Curve = Consol. settl. in % of ref. settlement.
O - Curve = Immediate settlements in % of the reference settlements.
T - Curve = Total settlements in % of the reference settlement.

4.3(c)

| 0 | 0.1 | 0.2 | 0.3 | 0.4 | 0.5 | 0.6 | 0.7 | 0.8 | 0.9 | 1.0 | 1.1 | 1.2 | 1.3 | 1.4 | 1.5 |
|---|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|

**I****I****I****I****I****I****I****I****I****I****I****I****I****I****


AVE DEGREE OF CONSOL. AND SETTLEMENT CURVES FOR POINT X = 40.00 FEET FROM CENTER LINE
INTERVAL BETWEEN 2 GRID LINES. EQ. 10%
ABSCISSA NUMBERS GIVE THE TIME IN WKS
THE TOTAL SETTL DUE TO REFERENCE LOAD IS = 0.372E+01 FT
U-CURVE = AVE. DEGREE OF CONSOL.
RELATIVE TO THE PORE PRESS DUE TO REF LOAD
C-CURVE = CONSOL. SETTL. IN % OF REF. SETTLEMENT
O-CURVE = IMMEDIATE SETTLEMENTS IN % OF THE REFERENCE SETTLEMENTS
T-CURVE = TOTAL SETTLEMENTS IN % OF THE REFERENCE SETTLEMENT

Fig. 4.3(d)

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AVE DEGREE OF CONSOL. AND SETTLEMENT CURVES FOR POINT X = 70.00 FEET FROM CENTER LINE
INTERVAL BETWEEN 2 GRID LINES. EQ. 10%
ABSCISSA NUMBERS GIVE THE TIME IN WKS
THE TOTAL SETTL DUE TO REFERENCE LOAD IS = 0.118E+01 FT
U-CURVE = AVE. DEGREE OF CONSOL.
RELATIVE TO THE PORE PRESS DUE TO REF LOAD
C-CURVE = CONSOL. SETTL. IN % OF REF. SETTLEMENT
O - CURVE = IMMEDIATE SETTLEMENTS IN % OF
THE REFERENCE SETTLEMENTS
T - CURVE = TOTAL SETTLEMENTS IN % OF
THE REFERENCE SETTLEMENT

Fig. 4.3(e)

0 0.1 0.2 0.3 0.4 0.5 0.6 0.7 0.8 0.9 1.0 1.1 1.2 1.3 1.4 1.5

AVE DEGREE OF CONSOL. AND SETTLEMENT CURVES FOR POINT X = 90.00 FEET FROM CENTER LINE
INTERVAL BETWEEN 2 GRID LINES = EQ. 10%
ABSCISSA NUMBERS GIVE THE TIME IN WKS
THE TOTAL SETTLE DUE TO REFERENCE LOAD IS = 0.539E+00 FT
U - CURVE = AVE. DEGREE OF CONSOL.
RELATIVE TO THE PORE PRESS DUE TO REF LOAD
C - CURVE = CONSOL. SETTLE. IN % OF REF. SETTLEMENT
O - CURVE = IMMEDIATE SETTLEMENTS IN % OF
THE REFERENCE SETTLEMENTS
T - CURVE = TOTAL SETTLEMENTS IN % OF
THE REFERENCE SETTLEMENT

g. 4.3(f)

0 0.1 0.2 0.3 0.4 0.5 0.6 0.7 0.8 0.9 1.0 1.1 1.2 1.3 1.4 1.5
Figure 4.4 Cross-Section and Idealized Soil Condition of the Second Sample Problem
**Borings 39 ST, 40 ST, VS 13, Route FAI 255, Section 82-8 HB, Station 267.00**

<table>
<thead>
<tr>
<th>Description</th>
<th>Depth</th>
<th>Water content (•), undrained strength from unconfined compression (●) and vane (○) tests, wet unit weight (+)</th>
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<tbody>
<tr>
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<td>01 02 03 04 05 06 07 08 09 10</td>
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<td>Gray alluvial clay</td>
<td>10</td>
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<tr>
<td>Blue, gray wet fine sand</td>
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</tr>
</tbody>
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Water content as drawal fraction, shear strength in ksf, unit weight in ksf.

**Figure 4.6 Boring Log**
Table 4.3 Data Card for Sample Problem 2 (Cont'd on next page)
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<th>Values</th>
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<td>5.6</td>
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<tr>
<td>35.0, 10.2</td>
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<td>78.2, 3.0</td>
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<td>81.2, 3.0</td>
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<td>93.2, 0.0</td>
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<td>81.2, 3.0</td>
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<td>93.2, 0.0</td>
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<td>1.15, 1.2, 0.8, 0.0, 57.5, 40.0, 0.0</td>
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<tr>
<td>5.6</td>
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<td>0.0, 10.2</td>
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<tr>
<td>93.2, 0.0</td>
</tr>
<tr>
<td>1.25, 0.8, 0.64, 40.0, 0.0</td>
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Abridged output for Sample Problem 2

THE PORE WATER PRESSURES ARE COMPUTED AT

<table>
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<th>YE/H</th>
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<th>0.200</th>
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<tr>
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<td>0.600</td>
<td>0.700</td>
<td>0.800</td>
<td>0.900</td>
</tr>
<tr>
<td>YE/H</td>
<td>1.000</td>
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<td>XT/W</td>
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<td>0.150</td>
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<td>0.309</td>
<td>0.372</td>
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THE PORE PRESSURES ARE INTERPOLATED AT

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<tbody>
<tr>
<td>XE/W=</td>
<td>0.500</td>
<td>0.600</td>
<td>0.700</td>
<td>0.800</td>
<td>0.900</td>
</tr>
<tr>
<td>XE/W=</td>
<td>1.000</td>
<td>1.100</td>
<td>1.200</td>
<td>1.300</td>
<td>1.400</td>
</tr>
<tr>
<td>XE/W=</td>
<td>1.500</td>
<td>1.600</td>
<td>1.700</td>
<td>1.800</td>
<td>1.900</td>
</tr>
<tr>
<td>XE/W=</td>
<td>2.000</td>
<td>2.100</td>
<td>2.200</td>
<td>2.300</td>
<td>2.400</td>
</tr>
<tr>
<td>XE/W=</td>
<td>2.500</td>
<td>2.600</td>
<td>2.700</td>
<td>2.800</td>
<td>2.900</td>
</tr>
<tr>
<td>XE/W=</td>
<td>3.000</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

ASSUMING COLLOCATION POLYNOMIALS OF DEGREE
2 BETWEEN THE LIMITS 0.000 AND 0.300
4 BETWEEN THE LIMITS 0.300 AND 0.650
3 BETWEEN THE LIMITS 0.650 AND 1.300
2 BETWEEN THE LIMITS 1.300 AND 3.000

THE SUBSOIL IS DESCRIBED BY THE FOLLOWING
PARAMETERS WHICH ARE GIVEN FOR THE UPPER LAYER
IN CASE OF STRATIFICATION

TOTAL THICKNESS H= 26.000 FEET
reference for X-COORD W= 100.000 FEET

LAYER INTERFACE IS 7.800 FT BELOW SURFACE
LOWER/UPPER PERMEABILITY, RK= 0.769
LOWER/UPPER COEF. OF CONSOLIDATION, RC= 0.500
LOWER/UPPER INITIAL VOID RATIO, REO= 1.318
LOWER/UPPER COMPRESSION INDEX, RCC= 1.867
LOWER RECOMPRESSION/UPPER RECOMPRESSION-
-INDEX ROC= \( 0.107 \)

SKEMPTON PORE PRESSURE COEFFICIENTS ARE
A= 0.50 AND B= 1.00

INITIAL VOID RATIO = 2.050

THE COMPRESSION INDEX IS = 0.7500E+00
RECOMPRESSION INDEX/CC ROC= 0.200
INITIAL EFFECTIVE P AND PRECOMPRESSION 
PSTRESSES PC AS USED IN THE COMPUTATIONS 

Y IN FT  P IN PSF  PC IN PSF 
0.000  104.00  1000.00  
2.600  104.00  1000.00  
5.200  208.00  1000.00  
7.800  286.00  1000.00  
10.400  364.00  864.00  
13.000  442.00  680.00  
15.600  520.00  520.00  
18.200  598.00  598.00  
20.800  676.00  676.00  
23.400  754.00  754.00  
26.000  858.00  858.00  

NOTE -P(1) AND PC(1) MAY HAVE BEEN CHANGED 
as compared to input values to avoid over flow 

COEFF CONSL-VERT FLOW IS CV= 0.2000E-01FT**2/DAY 
COEF OF CONSL-HORI-FLOW IS CH= 0.3000E-01FT**2/DAY 

THE DRAINAGE CONDITIONS ARE 
FREE DRAINAGE AT THE BOTTOM 

THE SHEAR STRENGTH CHARACTERISTICS OF 
THE SUB SOIL AS USED IN THE STABILITY ANALYSIS ARE 

DEPTH IN FEET  COHESION IN PSF  P-RATIO 
0.000  280.000  0.100  
2.600  220.000  0.100  
5.200  150.000  0.150  
7.800  150.000  0.150  
10.400  150.000  0.150  
13.000  150.000  0.150  
15.600  150.000  0.150  
18.200  150.000  0.150  
20.800  210.000  0.150  
23.400  360.000  0.100  
26.000  310.000  0.100  

REFERENCE LOAD 
**************

SKEMPTON PORE PRESSURE COEFFICIENTS ARE 
A= 0.50 AND B= 1.00 

THE LOAD CHARACTERISTICS ARE GIVEN BY 
THE UNIT WEIGHT GLOAD= 125.00 PCF 
THE COHESION , CLOAD= 1000.00 PSF 
THICKNESS OF THE DRAINAGE BLANKET YWM= 3.00 FT 
THE TANGENT OF THE ANGLE OF INTERNAL 
FRICITION TGPHI= 0.5770 
MINP= 5 COOR POINTS XINP/YINP 
0.00 FEET  10.20 FEET 
35.00 FEET  10.20 FEET 
78.20 FEET  3.00 FEET 
81.20 FEET  3.00 FEET 
93.20 FEET  0.00 FEET 

THE ACTUAL LOAD IS APPROXIMATED BY 6 LOADS 
OF EQ INTENSITY WHICH EXTEND FROM X=0 TO ALPHA(I)
IF ALPHA(I) IS NEGATIVE, THIS LOAD IS SUBTRACTED

\[
\begin{align*}
\text{ALPHA(1)} &= 89.800 \text{ FEET} \\
\text{ALPHA(2)} &= 82.200 \text{ FEET} \\
\text{ALPHA(3)} &= 70.700 \text{ FEET} \\
\text{ALPHA(4)} &= 60.500 \text{ FEET} \\
\text{ALPHA(5)} &= 50.300 \text{ FEET} \\
\text{ALPHA(6)} &= 40.100 \text{ FEET}
\end{align*}
\]

THE AVERAGE PORE PRESSURES, UAVER(I)
THE CONSOLIDATION SETTLEMENTS, SETRC(I) AND
THE TOTAL SETTLEMENTS, SETRT(I), AT XT(I) ARE

\[
\begin{array}{cccc}
\text{XT FEET} & \text{UAVER (PSF)} & \text{SETRC FT.} & \text{SETRT FT} \\
2.01 & 1221.99 & 3.401 & 3.401 \\
15.00 & 1220.05 & 3.396 & 3.396 \\
27.99 & 1199.55 & 3.347 & 3.347 \\
30.86 & 1188.66 & 3.321 & 3.321 \\
37.21 & 1146.45 & 3.218 & 3.218 \\
47.50 & 1001.05 & 2.855 & 2.855 \\
57.79 & 831.05 & 2.403 & 2.403 \\
64.14 & 708.77 & 2.094 & 2.094 \\
67.47 & 658.25 & 1.960 & 1.960 \\
85.06 & 361.09 & 1.175 & 1.175 \\
109.94 & 118.93 & 0.421 & 0.421 \\
127.53 & 71.97 & 0.262 & 0.262 \\
141.39 & 49.53 & 0.187 & 0.187 \\
215.00 & 6.15 & 0.025 & 0.025 \\
288.61 & 0.76 & 0.003 & 0.003
\end{array}
\]

THE NUMBER OF LIFTS IS NL = 3

THE AVAILABLE CONSTRUCTION TIME IS TA = 360. DAYS. TA IS NOT NEEDED IF NL = 1
PARAMETERS USED IN THE STABILITY ANALYSIS
DMAX = 4.000 DMIN = 1.000
NARC = 5NRAD = 6
DMAX, DMIN ARE THE MAX AND MIN STEP SIZES
USED IN THE SEARCH PROCEDURE
NARC= ONE = HALF THE NUMBER OF SUB ARCS
NRAD= NUMBER OF RADIi USED FOR EACH TRIAL CENTER
OF ARCS
THE FACTOR OF SAFETY AT TIME T = 0.
DAYS FOR LIFT 1 IS FS = 1.221
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 66.00 Y = 41.00 RADIUS = 61.80 IN FT

LOAD NO 1 APPLIED AT TL = 0 DAYS
*****************************************
SKEMPTON PORE PRESSURE COEFFICIENTS ARE
A = 0.50 AND B = 1.00

***************
THE LOAD CHARACTERISTICS ARE GIVEN BY
THE UNIT WEIGHT GLOAD = 125.00 PCF
THE COHESION, CLOAD = 1000.00 PSF
THICKNESS OF THE DRAINAGE BLANKET YWM = 3.00 FT
THE TANGENT OF THE ANGLE OF INTERNAL
THE REQUIRED SAFETY FACTOR IS FSI = 1.150
THE SPECIFIED PORTION OF SETLEMENT IS
EQUAL TO 0.000
IF 95% OF THE AVE PORE PRESSURE
AT THE TIME OF APPLICATION OF THIS LOAD
HAVE DISSIPATED AT 0.8000*15POINTS XT
THIS LIFT IS ASSUMED TO BE THE LAST ONE

THE FACTOR OF SAFETY AT TIME T = 0.1
DAYS FOR LIFT 1 WAS .GE.FS = 1.221
AS COMPARED TO THE REQU. FSI = 1.150

THE FACTOR OF SAFETY AT TIME T = 7.
DAYS FOR LIFT 2 IS FS = 1.036
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 43.00 RADIUS = 62.97 IN FT
THE FACTOR OF SAFETY AT TIME T = 14.
DAYS FOR LIFT 2 IS FS = 1.036
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 43.00 RADIUS = 62.97 IN FT
THE FACTOR OF SAFETY AT TIME T = 21.
DAYS FOR LIFT 2 IS FS = 1.037
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 43.00 RADIUS = 62.97 IN FT
THE FACTOR OF SAFETY AT TIME T = 28.
DAYS FOR LIFT 2 IS FS = 1.037
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 43.00 RADIUS = 62.97 IN FT
THE FACTOR OF SAFETY AT TIME T = 35.
DAYS FOR LIFT 2 IS FS = 1.038
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 43.00 RADIUS = 62.97 IN FT
THE FACTOR OF SAFETY AT TIME T = 42.
DAYS FOR LIFT 2 IS FS = 1.038
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 43.00 RADIUS = 62.97 IN FT
THE FACTOR OF SAFETY AT TIME T = 49.
DAYS FOR LIFT 2 IS FS = 1.039
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 43.00 RADIUS = 62.97 IN FT
THE FACTOR OF SAFETY AT TIME T = 56.
DAYS FOR LIFT 2 IS FS = 1.039
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 43.00 RADIUS = 62.97 IN FT
THE FACTOR OF SAFETY AT TIME T = 70.
DAYS FOR LIFT 2 IS FS = 1.040
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 43.00 RADIUS = 62.97 IN FT
THE FACTOR OF SAFETY AT TIME T = 84.
DAYS FOR LIFT 2 IS FS = 1.041
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 43.00 RADIUS = 62.97 IN FT
THE FACTOR OF SAFETY AT TIME T = 98.
DAYS FOR LIFT 2 IS FS = 1.042
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 43.00 RADIUS = 62.97 IN FT
THE FACTOR OF SAFETY AT TIME T = 112.
DAYS FOR LIFT 2 IS FS = 1.044
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 43.00 RADIUS = 62.97 IN FT
THE FACTOR OF SAFETY AT TIME T = 126.
DAYS FOR LIFT 2 IS FS = 1.045
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 140.
DAYS FOR LIFT 2 IS FS = 1.046
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 154.
DAYS FOR LIFT 2 IS FS = 1.047
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 168.
DAYS FOR LIFT 2 IS FS = 1.048
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 182.
DAYS FOR LIFT 2 IS FS = 1.050
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 200.
DAYS FOR LIFT 2 IS FS = 1.052
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 224.
DAYS FOR LIFT 2 IS FS = 1.054
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 248.
DAYS FOR LIFT 2 IS FS = 1.056
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 272.
DAYS FOR LIFT 2 IS FS = 1.058
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 296.
DAYS FOR LIFT 2 IS FS = 1.060
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 320.
DAYS FOR LIFT 2 IS FS = 1.054
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 308.
DAYS FOR LIFT 2 IS FS = 1.055
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 336.
DAYS FOR LIFT 2 IS FS = 1.057
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 364.
DAYS FOR LIFT 2 IS FS = 1.059
AS COMPARED TO THE REQU. FSI = 1.150
FS HAS BEEN OBTAINED FOR THE ARC WITH
X = 57.50 Y = 44.00 RADIUS = 63.42 IN FT
THE FACTOR OF SAFETY AT TIME T = 364.

EITHER THE FACTOR OF SAFETY, FS = 1.059
AND/OR THE SETTLEMENT, SETC(1) = 0.751 FEET
ARE LESS THAN SPECIFIED AT TIME T = 364 DAYS
WHICH IS GREATER THAN TA = 360 DAYS
*** THIS LIFT NO 1 IS, THEREFORE
CONSIDERED TO BE THE LAST ONE***

THE CONSOL. PROCESS

***********************

THE FOLLOWING INFORMATION IS OUT PUT
UAVE(X(1)),UAVE(X(2)),........,
= AVER DEGREES OF CONSOL. WITH RESPECT TP REF. LOAD
SETC(X(1)),SETC(X(2)),........,
= CONSOL. SETTLEMENTS
SETI(X(1)),SETI(X(2)),........,
= IMMEDIATE SETTLEMENTS
SETT(X(1)),SETT(X(2)),........,
= CONSOLI. + IMMEDIATE SETTLEMENTS
LAST TWO LINE ARE ONLY OUT PUT
IF SOIL IS PARTIALLY SATURATED (B.NE.1.)
THE POINTS X(I) IN FEET ARE AS FOLLOWS

0.000 80.000

T = 0.
DAYS IS THE TIME OF LOAD APPLICATION

T = 0. DAYS
<table>
<thead>
<tr>
<th>Time (DAYS)</th>
<th>Value 1</th>
<th>Value 2</th>
</tr>
</thead>
<tbody>
<tr>
<td>7</td>
<td>0.087</td>
<td>0.072</td>
</tr>
<tr>
<td>14</td>
<td>0.091</td>
<td>0.075</td>
</tr>
<tr>
<td>21</td>
<td>0.095</td>
<td>0.078</td>
</tr>
<tr>
<td>28</td>
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<td>35</td>
<td>0.102</td>
<td>0.084</td>
</tr>
<tr>
<td>42</td>
<td>0.106</td>
<td>0.087</td>
</tr>
<tr>
<td>49</td>
<td>0.109</td>
<td>0.090</td>
</tr>
<tr>
<td>56</td>
<td>0.112</td>
<td>0.092</td>
</tr>
<tr>
<td>$T$</td>
<td>Days</td>
<td></td>
</tr>
<tr>
<td>-----</td>
<td>------</td>
<td></td>
</tr>
<tr>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>70</td>
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<tr>
<td>126</td>
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<tr>
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<tr>
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<tr>
<td>196</td>
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</tbody>
</table>

<p>|   | 0.098 | 0.202 |
|   | 0.103 | 0.210 |
|   | 0.108 | 0.219 |
|   | 0.112 | 0.226 |
|   | 0.117 | 0.234 |
|   | 0.121 | 0.241 |
|   | 0.125 | 0.248 |
|   | 0.129 | 0.254 |
| 168| 0.137 |</p>
<table>
<thead>
<tr>
<th>T</th>
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<tr>
<td>0.613</td>
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<table>
<thead>
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<table>
<thead>
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<tr>
<td>0.663</td>
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<tbody>
<tr>
<td>0.202</td>
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<tr>
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<td>0.177</td>
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<tr>
<td>0.751</td>
<td>0.328</td>
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<table>
<thead>
<tr>
<th>T</th>
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<tbody>
<tr>
<td>0.224</td>
<td>0.182</td>
</tr>
<tr>
<td>0.771</td>
<td>0.337</td>
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</table>

<table>
<thead>
<tr>
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<tbody>
<tr>
<td>0.237</td>
<td>0.193</td>
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<tr>
<td>0.808</td>
<td>0.354</td>
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</table>

<table>
<thead>
<tr>
<th>T</th>
<th>504. DAYS</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td></td>
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</tbody>
</table>
AVE DEGREE OF CONSOL. AND SETTLEMENT CURVES FOR POINT X = 0.00 FEET FROM CENTER LINE
INTERVAL BETWEEN 2 GRID LINES = EQ. 10%
ABSCISSA NUMBERS GIVE THE TIME IN WKS
THE TOTAL SETTL DUE TO REFERENCE LOAD IS = 0.340E+01 FT
U - CURVE = AVE. DEGREE OF CONSOL.
RELATIVE TO THE PORE PRESS DUE TO REF LOAD
C - CURVE = CONSOL. SETT. IN % OF REF. SETTLEMENT

g. 4.7(a)

0.1 0.2 0.3 0.4 0.5 0.6 0.7 0.8 0.9 1.0 1.1 1.2 1.3 1.4 1.5
AVE DEGREE OF CONSOL. AND SETTLEMENT
CURVES FOR POINT X = 80.00 FEET FROM CENTER LINE
INTERVAL BETWEEN 2 GRID LINES = EQ. 10%
ABSCISSA NUMBERS GIVE THE TIME IN WKS
THE TOTAL SETTL DUE TO REFERENCE LOAD IS = 0.144E+01 FT
U = CUREV = AVE. DEGREE OF CONSOL.
RELATIVE TO THE PORE PRESS DUE TO REF LOAD
C = CURVE = CONSOL. SETTL. IN % OF REF. SETTLEMENT
4.7(b)

0 0.1 0.2 0.3 0.4 0.5 0.6 0.7 0.8 0.9 1.0 1.1 1.2 1.3 1.4 1.5

APPENDIX - A
PROGRAM MSAND
C FORT1 and FORT2 are internal files.
C INPUT and OUTPUT files must be specified by the USER.
C
*****************************************************************************
C
C THIS PROGRAM OPTIMIZES THE RATE AT WHICH A SPECIFIC HIGHWAY
C EMBANKMENT CAN BE CONSTRUCTED ON SOFT SUBSOIL. THIS PROBLEM
C INVOLVES THE COMPUTATION OF STRESSES AND PORE PRESSURES IN
C THE SUBSOIL, THE DISSIPATION OF THESE PORE PRESSURES, THE
C CORRESPONDING INCREASE IN SHEAR RESISTANCE AND THE STABILITY
C OF THE EMBANKMENT.
C
C THE EMBANKMENT LOAD WHICH IS ASSUMED TO ACT VERTICALLY,
C INDUCES PORE PRESSURES IN THE SUBSOIL WHICH ARE COMPUTED USING
C THEORY OF ELASTICITY AND SKEMPTON-PORE PRESSURE PARAMETERS
C A AND B. THESE PORE PRESSURES DISSIPATE ACCORDING TO THREE
C DIMENSIONAL CONSOLIDATION THEORY WHICH TAKES INTO ACCOUNT
C THE EFFECT OF GAS AND VARIABLE SOIL PARAMETERS. AS THE PORE
C WATER PRESSURE DISSIPATES THE EFFECTIVE STRESSES IN THE SUB-
C SOIL WILL INCREASE GIVING A SIMULTANEOUS INCREASE IN SHEAR
C RESISTANCE. SETTLEMENTS ARE COMPUTED FROM THE DISSIPATED PORE
C PRESSURES.
C
*****************************************************************************
Characte*50 INPUTFILE,OUTPUTFILE

REAL KVO,KHO

DIMENSION AX(5),JSP(10),MXE(4),MXT(4),MXS(4),OMEGA(40),OMED(40)
DIMENSION PHI(20),PHID(20),R(510),SETC(20),SETI(20),SETT(20)
DIMENSION SETRC(20),SETRI(20),SETRT(20),SPECS(10),SPECU(10)
DIMENSION T(150)
DIMENSION TB(50),TL(10),UA(220),UB(220),UC(220),UD(220),UAVE(20)
DIMENSION UAVER(20),XE(51),XME(660),XMT(100),XINP(20),XRP(20)
DIMENSION Y(11),YINP(20),YRP(20),RSP(100),UAVED(20)
DIMENSION SETR(20),ROW(100),KK(20),SYMB(4),XT(20),SV(11)
DIMENSION SVM(12),P(11),PC(11),PLOG(11),CO(11)
DIMENSION CP(11),SU(561),UAVEM(20),UM(220),UU(220),IDEN(10)
DIMENSION UE(40),UF(40)

EQUIVALENCE (UA(1),XME(1)),(UB(1),XME(221))
EQUIVALENCE (UC(1),XME(441)),(UD(1),XMT(1))

COMMON/ SAPID/ IOUTP,W,HH,GLOAD,CLOAD,NARC,NRAD
COMMON/ SADI/ LAYER,IBCV,HHE,M,N,IDC,NDR,ISM,SET(41)
COMMON/ SADIZ/ FIMPV,RC,RK,C,RO,RE,TA,ISP,IVAR
COMMON/ SACSE/ ROC,ROCL,SM,P,PC,PLOG,PO,PCO,IAV,IK,ISAT,AAV,AAS
COMMON/ SACO1/ AVOC,KVO,KHO,EOPUS,PU,SKHM,SKVM,C,NNN,ICOEF
COMMON/ SACO2/ PCV(10),CVIN(10),PCH(10),CHIN(10),ICV,KOUNT,HF
COMMON/ SADET/ XSTAB(51),YSTAB(11),DX,DY,YWM,TGPHI

DATA (TB(I),I=1,45)/
  1 0.,  7., 14., 21., 28., 35., 42., 49., 56.,
  2 70., 84., 98., 112., 126., 140., 154., 168., 196.,
  4 560., 616., 672., 728., 819., 910., 1001., 1092., 1274.,

C Specify the INPUTFILE and OUTPUTFILE Names.
INPUT=3
IOUTP=4

C WRITE (*,1132)
1132 FORMAT ("---Specify the Name of your INPUTFILE")
READ (*,1133)INPUTFILE
WRITE (*,1134)
1134 FORMAT ("---Specify the Name of the OUTPUTFILE")
READ (*,1133)OUTPUTFILE
FOPEN (UNIT=3,FILE=INPUTFILE,STATUS="OLD")
REWIND (3)
FOPEN (UNIT=4,FILE=OUTPUTFILE,STATUS="NEW",FORM="FORMATTED")
REWIND (4)
WRITE (*,1131)
1131 FORMAT ("Specify the No. of Symbols to be used in the plot")
C The Following symbols are proposed for the user to input.
C Blank = a Blank Space
C STAR = *
C GRID = 1
C SYMB (1) = U --For Ave. Deg. of Consolidation.
C 2 = C --For Consol. Settlement.
C 3 = O --For Imm.
C 4 = T --For Total
READ (*,*) MMM
WRITE (*,1130)
1130 FORMAT ("Specify Characters=Blank,STAR,GRID,(SYMB(I),1,MMM)")
READ (*,194)BLANK,STAR,GRID,(SYMB(I),1=1,MMM)

C C DETERMINATION OF MESH POINTS FOR FINITE DIFFERENCE SCHEME AND
C STABILITY ANALYSIS.
C MYE.LE.12,MHE.LE.40: Number of vertical and horizontal points
C in the finite difference scheme. HF=1.—for horizontal flow
C POR=Horizontal drainage distance/(XT(IEND)*W).
C POR—the distance to the zero pore pressure in H dirn from
C the center line. Put IPOR=1 (if POR is input by the user).
C Put POR=1.0 and IPOR=0 then the program will determine POR
C ****************************************
999 READ (INPUT,*)MYE,MHE,ITBL,ISP,HF,POR,IPOR
IF (ITBL.LE.0) ITBL=45
IF (ITBL.GT.45) ITBL=45
C MMM Total no. of SYMB (Symbols) to be used in the output
C IF (ISP.EQ.1) GOTO 9001
C Read no. of points for which output is required, if ISP=0.
C If ISP=1 (special points), output the data for points XT.
read (INPUT,*) JND
read (INPUT,*) (JSP(K),K=1,JND)
C LND = No. of lines to be printed.
C (i.e No. of weeks on the time axis in the output)
9001 continue
READ (INPUT,*)LND
C C GENERATE THE MATHEMATICAL MOLECULE FOR SIMPSONS OR TRAPEZOIDAL
C RULE IN VERTICLAL DIRECTION.
C CALL GENS (SV,MYE)
C C GENERATE VECTOR YE
C MT=MYE-1
D = MT
D = 1./D
DO 11 I = 1, MT
AI = I - 1
11 YE(I) = AI*D
YE(MYE) = 1.
IF (ISP.EQ.1) GOTO 1

READ NUMBERS OF EQUIDISTANT POINTS, MX, IN X-DIRECTION, NUMBER OF INTERVALS, NI, AND INTERVAL LIMITS AX(I), I = 1, NI.
THESE LIMITS ARE DIMENSIONLESS. TO GET THE CORRESPONDING VALUES IN FEET, THE AX(I) ARE MULTIPLIED BY THE REFERENCE VALUE W WHICH IS INPUT LATER.

READ (INPUT,* ) MX, NI
NIM = NI - 1
READ (INPUT,* ) (AX(I), I = 1, NI)

READ NUMBER OF UNEQUALLY SPACED POINTS IN EACH INTERVAL, MXT(I), GENERATE THE POINTS XT, IF ISP = 0

READ (INPUT,* ) (MXT(I), I = 1, NIM)
IEND = 0

GENERATE XT-S

DO 21 J = 1, NIM
MT = MXT(J)
ISTT = IEND + 1
IEND = IEND + MT
D = 2*MT
AS = AX(J + 1) + AX(J)
AD = AX(J + 1) - AX(J)
DO 22 I = ISTT, IEND
AI = 2*(IEND - I) + 1
22 XT(I) = (AD*COS(AI*3.14159265358979/D) + AS)/2.
21 CONTINUE
GOTO 2

READ SPECIAL POINTS IF ISP = 1
IEND must be at least 6 and 4 points be directly under the main embankment.

1 READ (INPUT,* ) IEND
READ (INPUT,* ) (XT(I), I = 1, IEND)
GOTO 42

Generate equidistant points in x-dirn including the limits AX(1) and AX(NI). Determine the number of XE-s in each interval MXE(I). If the left limit of the I-th interval coincides with an XE(K), the limit is considered in MXE(I).

2 MT = 1
J = 2
MM = MX - 1
D = MM
D = (AX(NI) - AX(1))/D
XE(1) = AX(1)
DO 23 I = 2, MM
XE(I) = XE(I-1) + D
IF (XE(I).LT.(AX(J)-0.001)) GO TO 23
MXX(J-1) = I-MT
MT=I
J=J+1
23 CONTINUE
XE(MX) = AX(NI)
MXX(J-1) = MX+1-MT

C

C INITIATE THE DETERMINATION OF MATRICES XMT, XME AND R
C
IEND=0
JEND=0
IRND=0
DO 41 J=1,NIM
C GENERATE MATRIX XMT FROM XT-S AND INVERT XMT
C
ISTT=IEND+1
IEND=IEND+MXX(J)
C
CALL MATR (ISTT,IEND,MXX(J),XT,AX(J),XMT)
CALL MINV (XMT,MXX(J),DETER)
C
C Generate matrix XME from XE-s
C
JSTT=JEND+1
JEND=JEND+MXX(J)
CALL MATR ( JSTT,JEND,MXX(J),XE,AX(J),XME)
C
C Post multiply the inverse of matrix XMT, which is stored in
C array XMT, by matrix XME, and store the result in matrix R
C starting with element R(IRST).
C
IRST=IRND+1
IRND=IRND+MXX(J)*MXX(J)
CALL MPRD (XMT,XME,R,MXX(J),MXX(J),MXX(J),1,1,IRST)
41 CONTINUE
42 AIEND=IEND
C
C NOTE THAT THE LAST VALUE OF IEND IS EQUAL TO THE TOTAL
C NUMBER OF XT-S.
C
C
C INPUT DATA FOR A SPECIFIC CASE.
C
C Read thickness of soil layer, H, Unit weight of the embankment load, GLOAD, Undrained strength of the embankment material, CLOAD, Reference length in H-dimn, W, Thickness of the drainage blanket, YWM, Tangent of angle of internal friction, TGPHI.
C If H=0.0 the program is terminated.
C
100 READ ( INPUT,* ) H,GLOAD,CLOAD,W,YWM,TGPHI
IF ( H.EQ.0.0.) GOTO 10000
IF ( H.EQ.99.9 ) GOTO 999
HH=H
C
C IDC=1 vertical flow only in all XT-s
C IDC=2 vertical + horizontal flow.
C
IDC=2
IF (HF.EQ.0.) IDC=1
C DETERMINE INCREMENTS IN VERTICAL AND HORIZONTAL DIR
C -CTION, DY AND DHX
45 DY=MEY-1
   DY=H/DY
   DYSQ=DY*DY
C
C Read drainage identifier IBCV
C IBCV=1 Impeded drainage at Y=H,
C IBCV=2 Free drainage at Y=H,
C IBCV=3 No drainage at Y=H
C Read location of interface in case of inhomogeneous soil,
C 4.LE.LAYER.LE.MYE-3
C
READ (INPUT,*),IBCV,LAYER
N=MHE-1
M=MEY-2
IF (IBCV.EQ.3) M=MEY-1
IF (IBCV.EQ.2) FIMPV=0.
IF (IBCV.GT.1) GO TO 4
C
C Read thickness of impeded layer,HI, and ratio of permeabilities,
C RKV=K(draining soil,vertical)/K(impedence layer,vertical)
C
READ (INPUT,*),HI, RKV
   CHIV=KV*HI/DY
   FIMPV=CHIV/(1.+CHIV)
4  IF (HF .EQ. 0.) GO TO 7
C Determine the horizontal grid point to be used in the finite
C difference scheme.
C
AI=MHE-1
DHX=POR*XT(IEND)*W/AI
IAI=AI
DO 25 I=1,IAI
   XET(I)=POR*(I-1)*XT(IEND)/AI
25 CONTINUE
   DXSQ=DHX*DHX
C Check for layer interface. Read ratio of permeabilities, RK=
C K(lower)/K(upper), and ratio of coeff. of consolidation, RC=
C CV(lower)/CV(upper), REO=EO(lower)/EO(upper)=Ratio of initial
C void ratios, RAV=AVO(lower)/AVO(upper)=Ratio of coeff. of
C compress., RCC=cc(lower layer)/cc(upper layer)=Ratio of
C compression indices. ROCL=cc(recompression,lower)/cc(upper)
C
7  IF (LAYER .LT. 3) GOTO 6
   READ (INPUT,*),RK,RC,REO,RAV,RCC,ROCL
IF (LAYER .GT. (MYE-3)) LAYER=0
C
C Read identifiers and parameters for the compressible layer
C IVAR=0 Const. soil parameters in the consolidation process.
C IVAR=1 Variable soil parameter in the consolidation process.
C ISAT=0 100% saturation.
C ISAT=1 Partial saturation.
C IAV=0 Const. coeff. of compressibility.
C IAV=1 Variable coeff. of compressibility.
C ICV=0 Vectors of coeff of consolidation are not input.
C ICV.GT.0 Vectors of coeff. of consol. are input. Variable
C CV and CH are obtained in subroutine COEFF by
C interpolation.
C IK=0 Const. coeff. of permeability.
C IK=1 Variable coeff. of permeability.
C
C EO Initial void ratio.
C KVO Initial vertical permeability.
C KHO Initial horizontal permeability.
C AVO Initial coeff of compressibility.
C A,B Skempton Pore pressure coefficients.
C CC Compression index of the virgin part of the E Vs LOG(P) curve.
C ROC (Recompression index)/CC in case of consolidation.
C GAMMA Buoyant unit weight for computing initial eff. stress.
C P(I) Initial effective stresses.
C PC(I) Precompression stresses.
C CV,CH Vertical and Horizontal coeff. of consol.
C S Degree of saturation.
C PU Initial Gas pressure.
C SKV,SKH Slopes of the void ratio Vs LOG(permeability) curve.
C
6     READ (INPUT,*) IVAR,IAV,ICV
     IF (ICV .NE. 0) IVAR=1
     READ (INPUT,*) EO,A
     ALPHG=1.
     B=1.
     IF (IAV .EQ. 1) GOTO 350
     READ (INPUT,*) AVO
     GOTO 563
C
C Read cc, ROC and GAMMA. If GAMMA.NE.0., Initial eff stresses,P,
C are computed and normal consol. is assumed. If GAMM=0., initial
C and precompression stresses are input.
C
350     READ (INPUT,*) CC,ROC,GAMMA
     IF (GAMMA .EQ. 0.) GOTO 561
C
C Initial eff stresses are computed. PO and PCO are averages
C
      AD=DY*GAMMA
      P(1)=AD
      PC(1)=AD
      PLOG(1)=0.
      PO=GAMMA*H/2.
      PCO=PO
      AI=0.
      DO 565 I=2,MYE
           AI=AI+AD
           P(I)=AI
           PC(I)=AI
           PLOG(I)=0.
      565 CONTINUE
       GO TO 563
C
C Initial eff. and precompression stresses are input.
C
561     READ (INPUT,*) P(1),PC(1)
      PO=SV(1)*P(1)
      PCO=SV(1)*PC(1)
      DO 562 I=2,MYE
       READ (INPUT,*) P(I),PC(I)
      PO=PO+SV(I)*P(I)
PCO = PCO + SV(I) * PC(I)
PLOG(I) = ALOG(PC(I) / P(I))

CONTINUE
IF (P(1) .EQ. 0.) P(1) = P(2)
IF (PC(1) .EQ. 0.) PC(1) = PC(2)
PLOG(I) = ALOG(PC(I) / P(I))

C If constant parameters are to be used in the cosol. process
C read CV, CH, AVO. If variable parameters are to be used, IF IVAR = 1
C read necessary coeffs.

562 IF (IVAR .EQ. 1) GOTO 564
READ (INPUT, *) CV, CH
GOTO 30
564 IF (ICV .EQ. 0) GOTO 572
DO 573 I = 1, ICV
573 READ (INPUT, *) PCV(I), CVIN(I), PCH(I), CHIN(I)
GOTO 574
572 READ (INPUT, *) KVO, KHO
574 IF (IAV .EQ. 1) GOTO 566
AVOC = AVO
PO = 0.
PCO = 0.
566 READ (INPUT, *) ISAT, IK
IF (ISAT .EQ. 0) GOTO 567
READ (INPUT, *) S, PU, HC, B
IF (PU .EQ. 0) PU = 2117. + 62.43 * H / 2.
EOP = EO * PU * (1. - S * (1. - HC))
567 IF (IK .EQ. 1) READ (INPUT, *) SKV, SKH
30 CONTINUE

C Read NC initial shear strengths CO(I) and C/PBAR-ratios
C at arbitrary depths Y(I). If NC .EQ. MYE, the depths are
C assumed to be equal to H*YE(I). If NC .LE. MYE the values
C of CO(J) and PC(J) at H*YE(J) are obtained by interpolation.
C The input values are not saved.

C IF (ISP .EQ. 1) GOTO 585
READ (INPUT, *) NC
DO 580 I = 1, NC
READ (INPUT, *) Y, UA(I), UB(I)
580 YRP(I) = Y / H
IF (NC .EQ. MYE) GOTO 581
CALL LAGR (YE, CO, MYE, 1, YRP, UA, NC)
CALL LAGR (YE, CP, MYE, 1, YRP, UB, NC)
GOTO 582
581 DO 583 I = 1, MYE
CO(I) = UA(I)
583 CP(I) = UB(I)
C
C Define the vectors XSTAB and YSTAB which are needed in the
C stability analysis.

C DO 584 I = 1, MX
584 XSTAB(I) = W * XE(I)
DX = XSTAB(2) - XSTAB(1)
C
DO 586 I = 1, MYE
586 YSTAB(I) = H*YE(I)
585 CONTINUE
C OUTPUT OF DATA INPUT SO FAR. FOR FORMAT STATEMENTS
C SEE END OF PROGRAM
C
C
WRITE (IOUTP,901)
WRITE (IOUTP,900)
WRITE (IOUTP,903)
WRITE (IOUTP,900)
WRITE (IOUTP,904)
WRITE (IOUTP,902)
WRITE (IOUTP,905) (YE(I),I=1,MYE)
WRITE (IOUTP,902)
WRITE (IOUTP,906) (XT(I),I=1,IEND)
WRITE (IOUTP,902)
IF (ISP.EQ.1) GOTO 53
WRITE (IOUTP,907)
WRITE (IOUTP,902)
WRITE (IOUTP,908) (XE(I),I=1,MX)
WRITE (IOUTP,909)
DO 51 I=1,NIM
MM=MXT(I)-1
51 WRITE (IOUTP,910) MM,AX(I),AX(I+1)
WRITE (IOUTP,902)
53 WRITE (IOUTP,901)
WRITE (IOUTP,902)
WRITE (IOUTP,913)
WRITE (IOUTP,914)H,W
WRITE (IOUTP,902)
IF (LAYER.LT.3) GOTO 54
AI=YE(LAYER)*H
WRITE (IOUTP,915) AI,RK,RC,REO
IF (IAV.EQ.0) GOTO 551
WRITE (IOUTP,715)RCC,ROCL
GOTO 552
551 WRITE (IOUTP,815) RAV
552 WRITE (IOUTP,902)
54 WRITE (IOUTP,916) A,B
IF (((1.-B).LT.0.00001) GOTO 55
WRITE (IOUTP,917) S,HC,PU
WRITE (IOUTP,902)
55 WRITE (IOUTP,918) EO
WRITE (IOUTP,902)
IF (IAV.EQ.1) GO TO 553
WRITE (IOUTP,818)AVO
WRITE (IOUTP,902)
GOTO 554
553 WRITE (IOUTP,919)CC,ROC
WRITE (IOUTP,819)
DO 555 I=1,MYE
AI=YE(I)*H
WRITE (IOUTP,719) AI,P(I),PC(I)
555 CONTINUE
WRITE (IOUTP,619)
WRITE (IOUTP,902)
554 IF (IVAR.EQ.1) GOTO 556
WRITE (IOUTP,934) CV,CH
GOTO 57
556 IF (ICV.EQ.0) GOTO 575
WRITE (IOUTP,820)
DO 576 I=1,ICV
WRITE(IOUTP,720)PCV(I),CVIN(I),PCH(I),CHIN(I)
GOTO 57
WRITE (IOUTP,920) KVO,KHO
IF (IK.EQ.1) WRITE (IOUTP,921)SKV,SKH
WRITE (IOUTP,902)
WRITE (IOUTP,922)
IF (IBCV.NE.1) GOTO 61
WRITE (IOUTP,926)HI,RKV
GOTO 64
IF (IBCV.EQ.3) GOTO 63
WRITE (IOUTP,927)
GOTO 64
WRITE (IOUTP,928)

WRITE (IOUTP,902)
IF (ISP.EQ.1) GOTO 587
WRITE (IOUTP,961)
DO 588 I=1,MYE
WRITE (IOUTP,962) YSTAB(I),CO(I),CP(I)
WRITE (IOUTP,900)

Define the modified molecules SVM in vertical direction.
Redefine ROCL to be the ratio of the Recompression index of the lower layer and the compression index of the virgin part of the lower layer.
Redefine also the parameters KKK,KIAV,NNN,FUP,FLO,SKVM,SKHM CCC,AAV,AAH and ICOEFF, which are needed in subroutines SETL and COEFF.
Redefine also PCV,PCH,CVIN,CHIN in case that ICV.NE.0

AI=1.+EO
AAV=AI/(62.42796*DYSQ)
IF (HF.EQ.0.) GOTO 521
AAH=AI/(62.42796*DXSQ)
KKK=MYE
KIAV=IAV+1
NNN=1
AD=H
IF (LAYER.GE.3) GOTO 524
DO 523 I=1,MYE
SVM(I)=SV(I)
GOTO 522
KKK=LAYER
CALL GENS(SVM,KKK)
MM=MYE-LAYER+1
CALL GENS(UA,MM)
II=LAYER
DO 525 I=1,MM
II=II+1
SVM(II)=UA(I)
AS=1.+EO*REO
IF (RCC.NE.0.) ROCL=ROCL/RCC
AD=H*YE(LAYER)
ICOEF=1
IF (IK.EQ.1) ICOEF=2+IAV
IF (ICV.EQ.0) GOTO 530
ICOEF=4
DO 529 I=1,ICV
PCV(I)=ALOG(PCV(I))
CVIN(I)=CVIN(I)/DYSQ
IF (HF.EQ.O.) GOTO 529
PCH(I)=ALOG(PCH(I))
CHIN(I)=CHIN(I)/DXSQ
CONTINUE
IF (IAV.EQ.1) GOTO 527

C Coeff. of compressibility is const (IAV=0)
FUP=AD*AVO/AI
IF (LAYER.GE.3) FLO=(H-AD)*AVO*RAV/AS
IF (IK.EQ.0) GOTO 520
SKVM=2.302585*AVO/SKV
IF (HF.EQ.O.) GOTO 520
SKHM=2.302585*AVO/SKH
GOTO 520

C Variable coeff. of compressibility IAV=1.
CC=0.4342945*CC
AVOC=CC/PCO
IF (PCO.GT.PO) AVOC=AVOC*(PCO/PO)**ROC
FUP=AD*CC/AI
IF (LAYER.GE.3) FLO=(H-AD)*CC*RCC/AS
IF (IK.EQ.0) GOTO 520
SKVM=CC/SKV
IF (HF.GT.O.) SKHM=CC/SKH
GOTO 520

REFERENCE LOAD.
******************************************************************************
C Read characteristics of the reference load.
C XINP,YINP -the coordinates of the polygon which describes
C the load.
C MINP- The no. of points.
C NS- The no. of strips by which the actual load is approximated.
C
520 READ (INPUT,*)MINP,NS
WRITE (IOUTP,935)
WRITE (IOUTP,916) A,B
WRITE (IOUTP,930) GLOAD,CLOAD,YWM,TGPHI,MINP
DO 101 I=1,MINP
READ (INPUT,*) XINP(I),YINP(I)
101 WRITE (IOUTP,931) XINP(I),YINP(I)

C Compute the pore water pressures at (XT(I),I=1,IEND)/YE(J),J=1,MYE)
CALL PORE (XINP,YINP,MINP,NS,XT,IEND,YE,MYE,UB,A,B)
C determine the horizontal distance from the Center line to the point
C where the pore pressure is 0.1% of the max pore pressure under the
C embankment. This is taken as a free drainage end.
CALL HDIST (UB,XT,IEND,ICV,CHIN,DXSQ,AAH,MHE,W,XET,IPOR,*
HF,MYE, POR)
IPOR=1

C Compute the average pore pressures UAVER(I),I=1,IEND for the
C reference load. Determine final consolidation settlements,SETREC(I),
C Immediate settlements, SETRI(I), and Total settlements (SETRT(I)), C I=1, IEND for the reference load.

II = 0
DO 501 I = 1, IEND
UAVER(I) = 0.
DO 102 J = 1, MYE
II = II + 1
UU(II) = UB(II)
UAVER(I) = UAVER(I) + UB(II) * SV(J)
501 CONTINUE

CALL SETL (UB, SETRC, IEND, KKK, MYE, I, FUP, FLO, KIAV)
IF (B.NE.1.) GOTO 513
DO 514 I = 1, IEND
SETRT(I) = SETRC(I)
514 SETRI(I) = 0.
GOTO 515
513 FAC = 1./B
CALL SETL (UB, SETRT, IEND, KKK, MYE, FAC, FUP, FLO, KIAV)

C Define initial parameters.
C TL(LIFT) Time of LIFT-th load application.
C TB(ITB) Time from TL(LIFT) till TL(LIFT+1) given in DATA statement.
C Y(IT) Time from first load application, T(IT) = TL(LIFT) + TB(ITB)

515 WRITE (IOUTP, 932)
DO 71 I = 1, IEND
AI = W * XT(I)
WRITE(IOUTP, 933) AI, UAVER(I), SETRC(I), SETRT(I)
71 CONTINUE

WRITE (IOUTP, 900)
ISUM = MYE * IEND
LIFT = 1
LL = LIFT + 1
ITB = 1
IT = 1
TL(1) = 0.
TB(1) = 0.
T(1) = 0.

C Read no. of lifts, NL. If computation for special points is required C (ISP=1), read also NL times of load application, TL.
C NLS is defined for checking purposes at the end of the program C IDEN(I).LT.0, Pore pressures due to the first load are set C equal zero.
C IDEN(I)=0, , I=1,NL pore pressures due to the I-th load are computed by means of subroutine PORE.
C IDEN(I)=1, I=1,NL Pore pressures due to the I-th load are set equal to those due to the REFERENCE load.

READ (INPUT, *) NL, (IDEN(I), I=1, NL)
NLS = NL
WRITE (IOUTP, 936) NL
WRITE (IOUTP, 902)
IF (ISP.EQ.0) GOTO 103
READ (INPUT, *) (TL(I), I=1, NL)
WRITE (IOUTP, 937)
DO 106 I=1, N
106 WRITE (IOUTP,938) I, TL(I)
     WRITE (IOUTP,902)
     T(I)=TL(I)+TB(I)
     GOTO 105

C C FIRST LOAD.
C **********************************************************************
C Read characteristics of the first load, also
C TA  Available construction time.
C SPECS  Specified settlement for the first lift.
C SPECU  When the NOT DISSIPATED average pore pressures
C become less than 5% of the total average pore pressure
C at the time of load application at IEND*SPECU points,
C the subsequent loads are disregarded.
C FSI  Factor of safety for the first lift.
C DMAX  max. interval used in the search for the minimum FS
C DMIN  Corresponding minimum interval.
C ZZ  Distance between the maximum YINP and the minimum
C permissible YC during the search procedure.
C NARC  One-half the no. of subarcs used in subroutine DETFS
C NRAD  No. of trial arcs used at each center YC,YC
C IAB=0  Use A and B as defined earlier.
C IAB.NE.0  Read new values of A and B.

103 READ (INPUT,*) FSI,SPECS(1),SPECU(1),TA,DMAX,DMIN,XC,YC,ZZ
       READ (INPUT,*) NARC,NRAD
       IF (DMAX.GT.DMIN) GOTO 401
       DMAX=H/2.
       DMIN=H/20.

401 WRITE (IOUTP,940) TA
       WRITE (IOUTP,840) DMAX,DMIN,NARC,NRAD
105 READ (INPUT,*) MINP,NS,IAB
      DO 506 I=1,MINP
506 READ (INPUT,*)XINP(I),YINP(I)
      IF (IAB.NE.0) READ (INPUT,*) A,B
      IF (ISP.EQ.1) GOTO 107

C C CALL SUBROUTINES FOR STABILITY ANALYSIS
C
      CALL INIT (XINP,YINP,MINP,XC,YC,YY,ZZ,DMIN)
      CALL GAIN (UA,R,SU,MYE,MXT,MXE,MYE,CO,CP,1)
      CALL STAB (XC,YC,RR,XINP,YINP,MINP,MX,MYE,SU,FS,DMAX,DMIN,YY)
      DO 570 I=1,ISUM
570 UA(I)=0.

C WRITE (IOUTP,942) T(IT),LIFT,FS,FSI
      WRITE (IOUTP,842) XC,YC,RR
      IF (FS.GE.FSI) GOTO 107
      WRITE (IOUTP,943)

C Since Branching to the input of a new case might result in the
C reading of the following load characteristics instead of the
C data for a new run, the program is terminated here.
C
      GOTO 10000
107 WRITE (IOUTP,902)

C C RESIDUAL PORE PRESSURES
C Read index IRP and, for IRP.\(\neq 0\) residual pore pressures.
C
C IRP=0 No residual pore pressures.
C IRP=1 Residual pore pressures at XT/YE are input.
C IRP=2 Residual pore pressures at points other than XY/YE
C are input.

C READ (INPUT,*), IRP
   IF (IRP-1) 109, 110, 111
C
C NO RESIDUAL PORE PRESSURES
C
109 DO 112 J=1, ISUM
112 UC(J)=0.
   GO TO 108
C
C Residual pore pressures at points XT/YE. Input is columnwise
C with each input card containing ten values
C
110 READ (INPUT,*), (UC(I), I=1, ISUM)
   WRITE (IOUTP, 944) (YE(J), J=1, MYE)
   II=1-MYE
   IJ=0
   DO 113 I=1, IEND
   II=II+MYE
   IJ=IJ+MYE
113 WRITE (IOUTP, 945) I, XT(I), (UC(J), J=11, IJ)
   GO TO 108
C
C Residual pore pressures at points other than XT/YE. Residual
C pore pressures at XT/YE are obtained by interpolation. The
C input values are not saved.
C
111 WRITE (IOUTP, 946)
   I=1
   J=1
   READ (INPUT,*), X, Y, UA(1), COUNT
   WRITE (IOUTP, 947) X, Y, UA(1)
   XRP(1)=X/W
   YRP(1)=Y/H
   IF (COUNT.EQ.0.) GO TO 115
   DO 114 J=1, ISUM
114 UC(J)=UA(1)
   GO TO 108
115 READ (INPUT,*), X, Y, U, COUNT
   WRITE (IOUTP, 947) X, Y, U
   X=X/W
   Y=Y/H
   IF (ABS(X-XRP(I)).LT.0.00001) GO TO 116
   IJ=(I-1)*MYE+1
C
C CALL LAGR (YE, UC, MYE, IJ, YRP, UA, J)
   I=I+1
   XRP(I)=X
   J=0
116 J=J+1
   YRP(J)=Y
UA(J)=U
IF (COUNT.EQ.0.) GO TO 115
IJ=(I-1)*MYE+1
CALL LAGR (YE,UC,MYE,IJ,YRP,UA,J)
IJ=I-IEND
DO 117 JJ=1,MYE
II=JJ-MYE
IJ=IJ+IEND
DO 118 J=1,1
II=II+MYE
UA(J)=UC(IJ)
117 CALL LAGR (XT,UD,IEND,IJ,XRP,UA,I)
IJ=0
DO 119 JJ=1,IEND
II=JJ-IEND
DO 119 J=1,MYE
IJ=IJ+1
II=II+IEND
UC(IJ)=UD(IJ)
119 CONTINUE

C Write load characteristics and compute average pore pressure and
C immediate settlements. rewind tape 1 for storage purposes.
C
108 REWIND 1
C
WRITE (IOUTP,900)
WRITE (IOUTP,939) LIFT,TL(LIFT)
WRITE (IOUTP,916) A,B
WRITE (IOUTP,930) GLOAD,CLOAD,YWM,TGPHI,MINP
DO 120 I=1,MINP
120 WRITE (IOUTP,931) XINP(I),YINP(I)
WRITE (IOUTP,902)
C
IF (IDEN(1)) 73,74,516
74 IDEN(1)=MINP
73 CALL PORE (XINP,YINP,IDEN(1),NS,XT,IEND,YE,MYE,UB,A,B)
IF (HF.EQ.0.) GOTO 516
IF (IP0R.EQ.1) GOTO 516
C
C Find the horizontal distance to the point where the pore pressure is
C 0.1% of the maximum pore pressure under the embankment. This is
C considered as the drainage end in the horizontal direction.
C
CALL HDIST (UB,XT,IEND,ICV,CHIN,DXSQ,AAH,MHE,W,XET,IPOR,
* HF,MYE,POR)
516 IF (ISP.EQ.1) GO TO 510
WRITE (IOUTP,941) FS,I,SPEC(LIFT),SPECU(LIFT),IEND
WRITE (IOUTP,960) TL(LIFT),LIFT,FS,FSI
WRITE (IOUTP,902)
C
C Compute immediate settlements, SETI, if B.NE.1 compute
C pore pressures UAVED from UB, if IRP=0 or from UB+UC,
C if IRP.NE.0, write information on tape 1.
C
510 IF (B.NE.1.) GO TO 402
DO 403 I=1,IEND
403 SETI(I)=0.
GO TO 405
402 CALL SETL (UB,SETC,IEND,KKK,MYE,1.,FUP,FLO,KIAV)
AI=1./B
CALL SETL (UB, SETT, IEND, KKK, MYE, AI, FUP, FLO, KIAV)
DO 404 I=1, IEND
404 SETI(I)=SETT(I)-SETC(I)
405 II=0
DO 121 I=1, IEND
SETC(I)=0.
SETT(I)=SETI(I)
UAVE(I)=0.
UAVED(I)=0.
UAVEM(I)=0.
WRITE (1) SETI(I), SETC(I), SETT(I), UAVE(I)
DO 122 J=1, MYE
II=II+1
UD(II)=UB(II)+UC(II)
UAVED(I)=UAVED(I)+UD(II)*SV(J)
UM(II)=UB(II)/B+UC(II)
UAVEM(I)=UAVEM(I)+UM(II)*SV(J)
UC(II)=UB(II)
122 UB(II)=UD(II)
121 CONTINUE
C Determine soil parameters in a form suitable for the subroutine DISP. Only one OMEGA- and ONE PHI- element for the case of constant soil parameters and full saturation.
C
if (IEND.GT.5) goto 1241
CALL LAGR (XET, UE, N, 1, XT, UAVEM, IEND)
CALL LAGR (XET, UF, N, 1, XT, UAVED, IEND)
go to 1242
1241 call LINT (XET, UE, N, N, XT, UAVEM, IEND)
call LINT (XET, UF, N, N, XT, UAVE, IEND)
1242 IF (IVAR.NE.0) GO TO 124
PHI(I)=CV/DYSQ
OMEGA(I)=0.
IF (HF.GT.0.) OMEGA(I)=CH/DXSQ
GOTO 123
124 KOUNT=1
CALL COEF (UAVED, UAVED, OMEGA, PHI, 1, 1, OMED, PHID, IEND)
IF (HF.EQ.0.) GO TO 125
CALL COEF (UE, UF, OMEGA, PHI, 1, 2, OMED, PHID, N)
125 KOUNT=0
C
C CONDITIONS IN THE CONSOLIDATION PROCESS.
C
123 CALL DISP (UB, 5, OMEGA, PHI, 0., UAVE, 1, MYE, IEND, XT, SV)
C
C SECOND OR FOLLOWING LOAD
C
C Read characteristics of next load depending on the value of LIFT+1.
C
Define the index ISTAB.
C
ISTAB=0 Determine the factor of safety.
C
ISTAB=1 NO stability analysis. ISTAB is set equal to 1 if pore pressures at special points are to be computed
C
ISP=1
C FSI=specified factor of safety for the LL-th load distribution.
C SPECS= specified fraction of the consolidation settlement.
C SPECU-- if an average degree of consol. of 0.95 due to the
lift-th load is obtained at SPECU(LIFT)*IEND points XT without
a sufficient factor of safety for the LL-th load the LIFT-th
load is taken to be the last load and NL is set NL=LIFT.
TMIN first stability analysis for the LL-th load will be
done. for TD.GE.TMIN days after application of the LIFT-th load.
XC,YC = coord of the center of the first trial arc.
If IAB.NE.O new pore pressure parameters A and B are input.

128 IF (LL.GT.NL) GOTO 129
READ (INPUT,*), MINP,NS,IAB
DO 130 I=1,MINP
130 READ (INPUT,*), XINP(I),YINP(I)
IF (IAB.NE.O) READ (INPUT,*), A,B
ISTAB=ISP
IF (ISP.EQ.1) GO TO 518
READ (INPUT,*), FSI,SPECS(LL),SPECU(LL),TMIN,XC,YC,ZZ
CALL INIT (XINP,YINP,MINP,XC,YC,YY,ZZ,DMIN)
GO TO 129
518 TSTEP=TL(LL)-TL(LIFT)
129 ITB=ITB+1
IF (ITB.GT.ITBL) GO TO 200

Compute pore pressures at time T(IT). Determine variable soil
parameters. Compute consol. settlements. Perform stability
analysis.

TD=TB(ITB)
IT=IT+1
IF (ISP.EQ.0) GOTO 131
IF (LL.GT.NL) GO TO 131
IF (TSTEP.LT.TD) TD=TSTEP
131 T(IT)=TTL(LIFT)+TD
CALL DISP (UB,2,OMEGA,PHI,TD,UAVE,LIFT,MYE,IEND,XT,SV)

Vector UB contains the pore pressures at time T(IT). Vector UVAE
contains the average pore pressures as computed from UB.
Vector UA contains the dissipated pore pressures up to time T(IT).

IF (IVAR.EQ.0) GO TO 232
IF (IEND.GT.5) go to 2321
CALL LAGR (XET,UF,N,1,XT,UAVE,IEND)
go to 2322
2321 call LINT (XET,UF,N,N,XT,UAVE,IEND)
2322 CALL COEF (UAVEM,UAVE,OMEGA,PHI,3,1,OMED,PHID,IEND)
CALL COEF (UE,UF,OMEGA,PHI,3,2,OMED,PHID,N)
CALL DISP (UB,3,OMED,PHID,TD,UAVE,LIFT,MYE,IEND,XT,SV)
232 IF (LL.GT.NL) GO TO 133
IF (ISTAB.EQ.1) GOTO 133
IF (TD.LT.TMIN) GO TO 133
DO 72 J=1,ISUM
UA(J)=UM(J)-UB(J)
72 CONTINUE
CALL GAIN (UA,R,SU,MYE,MXT,MXE,MX,NIM,CO,CP,0)
CALL STAB (XC,YC,RR,XINP,YINP,MINP,MX,MYE,SU,FS,DMAX,DMIN,YY)
IF (FS.LT.FSI) GOTO 75
ISTAB=1
WRITE (IOUTP,902)
75 WRITE (IOUTP,942) T(IT),LL,FS,FSI
WRITE (IOUTP,842) XC,YC,RR
Compute settlements and average degrees of consolidation.

Count at how many points XT the degree of consolidation as compared to the pore pressures at the time of the last load application is greater than 95%.

```
C
133  UCHEK=0.
   DO 132 J=1,ISUM
   UA(J)=UD(J)-UB(J)
   CONTINUE
   CALL SETL (UA,SETC,IEND,KKK,MYE,1.,FUP,FLO,KIAV)
   DO 134 I=1,IEND
   SETT(I)=SETI(I)+SETC(I)
   IF (UAVE(I).LT.(0.05*UAVED(I))) UCHEK=UCHEK+1.
   UAVE(I)=(UAVED(I)-UAVE(I))/UAVE(I)
   WRITE (1) SETI(I),SETC(I),SETT(I),UAVE(I)
   CONTINUE

C
In the following checks are made for the no. of lift, whether
the FS is sufficient for the next lift, whether a specified amount
of settlement at point XT(1) has already occurred, whether the
available construction time TA has been passed, and whether an
average degree of consol. relative to the pore pressures at the time
of load application of 95% has been reached at .GE.SPECU(LIFT)*IEND
points XT

C
IF (LIFT.EQ.NL) GOTO 129
IF (ISP.EQ.0) GO TO 135
IF (TD.EQ.TSTEP) go to 137
GO TO 129
135 IF (ISTAB.EQ.0) GO TO 138
IF (SETC(1).GT.SPECS(LIFT)*SETRC(1)) GO TO 137
138 IF (T(IT).GT.TA) GO TO 139
IF (UCHEK.LT.AIEND*SPECU(LIFT)) GO TO 129
NL=LIFT
III=UCHEK+0.1
WRITE (IOUTP,948) III,IEND,TL(LIFT),LIFT
GO TO 129
C
139 NL=LIFT
```
This page is left blank intentionally.
WRITE (IOUTP, 949) FS, SETC(I), T(IT), TA, LIFT
GO TO 129

C

LIFT=LL
LL=LIFT+1
IT8=1
TL(LIFT)=T(IT)
IT=IT+1
T(IT)=TL(LIFT)
WRITE (IOUTP, 900)
WRITE (IOUTP, 939) LIFT, TL(LIFT)
WRITE (IOUTP, 916) A, B
WRITE (IOUTP, 930) GLOAD, GLOAD, YWM, T0PHI, MINP
DO 140 I=1, MINP
140 WRITE (IOUTP, 931) XINP(I), YINP(I)
WRITE (IOUTP, 902)

C

compute pore pressures due to new load. Compute imm. settlements, SETI, if B.NE. 1. Note that the consol. settlements, SETC and the ave. degree of consol are the same as computed at time T(IT-1)=TL(LIFT).

C

Note pore pressures due to the load addition are neglected in the computation of imm. settlements. Since zero swelling is assumed, negative pore pressures after load application are set equal to zero. This means that negative pore pressures due to load removal are considered only in a magnitude equal to the not dissipated pore pressures just before load removal.

C

IF (IDEN(LIFT).EQ.0) GO TO 76
DO 77 I=1, ISUM
UA(I)=UU(I)
77 CONTINUE
GO TO 78

C

CALL PORE (XINP, YINP, MINP, NS, XT, IEND, YE, MYE, UA, A, B)

C

IF (ISP.EQ.1) GO TO 511
WRITE (IOUTP, 941) FSI, SPECS(LIFT), SPECU(LIFT), IEND
WRITE (IOUTP, 960) TL(LIFT), LIFT, FS, FSI
WRITE (IOUTP, 902)

511 IF ((1.-B).LT.0.00001) goto 143
CALL SETL (UA, SETRI, IEND, KKK, MYE, 1., FUP, FLO, KIAV)
AI=1./B
CALL SETL (UA, SETT, IEND, KKK, MYE, AI, FUP, FLO, KIAV)
do 141 I=1, IEND
SETRI(I)=SETR(I)-SETR(I)
IF (SETRI(I).GT. SETI(I)) SETI(I)=SETRI(I)
SETR(I)=SETR(I)+SETC(I)
141 CONTINUE

C

II=0
DO 144 I=1, IEND
WRITE (1) SETI(I), SETC(I), SETT(I), UAVE(I)
UAVE(I)=0.
UAVED(I)=0.
UAVEM(I)=0.
DO 144 J=1, MYE
II=II+1
UT=UB(II)+UA(I)-UC(II)
IF (UT.LT.0.) UT=0.
UAVE(I)=UAVE(I)+UT*SV(J)
144 CONTINUE
UB(II) = UT - UB(II)
UC(II) = UA(II)
UD(II) = UD(II) + UB(II)
UAVED(II) = UAVED(II) + UD(II) * SV(J)
UM(II) = UD(II) + (1. / B - 1.) * UA(II)
UAVED(II) = UAVED(I) + UM(II) * SV(J)

CONTINUE
LLL = LIFT - 1
CALL DISP (UB, 3, OMEGA, PHI, TD, UAVE, LLL, MYE, IEND, XT, SV)
IF (IVAR. EQ. 0) GO TO 244
IF (IEND. GT. 5) goto 1444
CALL LAGR (XET, UF, N, 1, XT, UAVE, IEND)
CALL LAGR (XET, UE, N, 1, XT, UAVEM, IEND)
goto 1445
call LINT (XET, UF, N, N, XT, UAVE, IEND)
call LINT (XET, UE, N, N, XT, UAVEM, IEND)
call COEF (UAVED, UA, OMEGA, PHI, 1, 1, OMED, PHID, IEND)
call COEF (UE, UF, OMEGA, PHI, 1, 2, OMED, PHID, N)
244 CALL DISP (UB, I, OMEGA, PHI, O., UAVE, LIFT, MYE, IEND, XT, SV)
GO TO 128

RETURN
128 CALL LINT (XET, UF, N, N, XT, UAVE, IEND)
218 DO 219 K = 1, JND
K = JSP(I)
218 XRP(I) = W * XE(K)

Determine indices MXS(I), I = 1, NIM and matrix RSP which is a sub
matrix of matrix R. Matrix RSP is needed when interpolation between
points XT is done to get the information required at points XRP/W.

IRE = 0
IRS = 0
K = 1
MMM = MXE(1)
MT = MXT(1)
DO 210 JJ = 1, NIM
MXS(JJ) = 0
J = 1
DO 211 I = 1, MX
IF (JSP(K). EQ. 1) GO TO 214
211 DO 212 J = 1, MT
IRS = IRS + 1
IRE = IRE + 1
212 RSP(IRS) = R(IRE)
C
MXS(JJ)=MXS(JJ)+1
IF (K.EQ.JND) GOTO 213
K=K+1
215 IF (MMM.NE.I) GO TO 211
JJ=JJ+1
MT=MXT(JJ)
MMM=MMM+MXE(JJ)
211 CONTINUE
C
C CASE THAT ISP=1
C
217 JND=IEND
DO 219 I=1,JND
219 XRP(I)=W*XT(I)
C
213 WRITE (IDUTP,951) (XRP(I),I=1,JND)
WRITE (IDUTP,900)
DO 250 I=1,JND
250 XRP(I)=XRP(I)/W
C
MMM=2
IF (B.NE.1.) MMM=4
MT=MMM*JND
C
C Read UAVE, SETC and SETT, given at points XT from TAPE 1. If
C ISP=1, this is the information to be output. IF ISP=0,
C perform interpolation using matrix R5P. Output the
C information and store it on TAPE 2 for later plots for times
C T(J), J=1,IT. Use UA and UB for temporary storage.
C
DO 220 J=1,IT
II=1
DO 221 I=1,IEND
READ (1) AI,UA(II+1),AA,UA(II)
IF ((1.-B).LT.0.00001) GOTO 224
UA(II+2)=AI
UA(II+3)=AA
224 II=II+MMM
221 CONTINUE
IF (ISP.EQ.1) GO TO 225
C
C Interpolation for points XRP(I)/W by means of matrix multi-
C plication. Information for points XT is in UA. Interpolation
C for points XRP(I)/W is stored in UB
C
IUBE=0
IUND=0
IRND=0
DO 216 JJ=1,NIM
IUBS=IUBE+1
IUBE=IUBE+MMM*MXT(JJ)
IF (MXS(JJ).EQ.0) GO TO 216
IUST=IUND+1
IUND=IUND+MMM*MXS(JJ)
IRST=IRND+1
IRND=IRND+MXT(JJ)*MXS(JJ)
CALL MPRD (UA,RSP,UB,MMM,MXT(JJ),MXS(JJ),IUBS,IRST,IUST)
CONTINUE
GO TO 226

DEFINE UB FOR CASE OF ISP=1

DO 222 I=1,MT
UB(I)=UA(I)

IF (J.EQ.1) GO TO 227
IF (T(J).NE.T(J-1)) GO TO 228
WRITE (IOUTP,952) T(J)
WRITE (IOUTP,953) T(J)

DO 223 K=1,MMM
WRITE (IOUTP,951) (UB(I), I=K, MT, MMM)
CONTINUE

WRITE (2) (UB(I), I=1,MT)

CONTINUE

REWIND 1
REWIND 2

PLOTTING ROUTINE
**********************************************************************

KEND=T(IT)/7.+1.001
IF (LND.GT.KEND) LND=KEND

Compute the reference settlement for points XRP from total settlements SETRT at points XT.

IF(ISP.EQ.0) GO TO 297
DO 298 I=1,IEND
SETR(I)=SETRT(I)
GO TO 299

IUBE=0
IUND=0
IRND=0
DO 406 I=1,NIM
IUBS=IUBE+1
IUBE=IUBE+MXT(I)
IF (MXS(I).EQ.0) GO TO 406
IUST=IUND+1
IUND=IUND+MXS(I)
IRST=IRND+1
IRND=IRND+MXT(I)*MXS(I)
CALL MPRD (SETRT, RSP, SETR, 1, MXT(I), MXS(I), IUBS, IRST, IUST)
CONTINUE

DO 300 J=1,JND
JS=(J-1)*MMM+1
WRITE TITLE FOR J-TH PLOT
X = XRIP(J) * W
WRITE (IDUTP, 901)
WRITE (IDUTP, 954) X, SETR(J), SYMB(1), SYMB(2)
IF (MMM.EQ. 4) WRITE (IDUTP, 955) SYMB(3), SYMB(4)
WRITE (IDUTP, 900)

C
REWIND 2
READ (2) (UB(I), I=1, MT)
C
C GENERATE FIRST LIN TO BE PRINTED
C
LOUT = T(1) / 7. + 0.1
JT = 2
K = T(2) / 7. + 0.1
DO 301 I = 1, 76
301 ROW(I) = STAR
DO 302 I = 6, 71, 5
302 ROW(I) = GRID
C
JJ = JS
II = 50. * UB(JJ) + 1.5
ROW(II) = SYMB(1)
C
DO 303 I = 2, MMM
JJ = JJ + 1
II = 50. * UB(JJ) / SETR(J) + 1.5
303 ROW(II) = SYMB(1)
C
C Write first line and clear ROW(I) afterwards.
C
WRITE (IDUTP, 957)
WRITE (IDUTP, 956) LOUT, (ROW(I), I = 1, 76)
DO 304 I = 2, 75
304 ROW(I) = BLANK
ROW(1) = STAR
ROW(76) = STAR
MK = MMM
C
C Determine the following to be printed.
C
DO 306 L = 2, LND
LOUT = LOUT + 1
IF (K.EQ. LOUT) GO TO 305
WRITE (IDUTP, 956) LOUT, (ROW(I), I = 1, 76)
GO TO 306
305 JT = JT + 1
IF (JT.GT. IT) JT = 1
K = T(JT) / 7. + 0.1
C
C Read data from TAPE and determine symbols to be printed
C
309 READ (2) (UB(I), I = 1, MT)
JJ = JS
IKK = MK - MMM + 1
II = 50. * UB(JJ) + 1.5
ROW(II) = SYMB(1)
KK(IKK) = II
C
DO 307 I = 2, MMM
JJ = JJ + 1
IKK=IKK+1
II=50. *UB(JJ)/SETR(J)+1.5
ROW(II)=SYMB(I)
KK(IKK)=II

307 CONTINUE

IF (K.NE.LOUT) GO TO 310
JT=JT+1
K=T(JT)/7.+0.1
MK=MK+MMM
GO TO 309

Print the L-th line and BLANK out the second through 75-th element.

310 WRITE (IOUTP,956) LOUT,(ROW(I),I=1,76)
DO 308 I=1,MK
II=KK(I)
308 ROW(II)=BLANK
ROW(1)=STAR
ROW(76)=STAR
MK=MMM
306 CONTINUE

Generate and print last line

DO 311 I=1,76
311 ROW(I)=STAR
DO 312 I=6,76,5
312 ROW(I)=GRID
LOUT=LOUT+1
WRITE (IOUTP,956) LOUT,(ROW(I),I=1,76)
300 CONTINUE

IF (NL.NE.NLS) GOTO 10000
GO TO 100

FORMAT STATEMENTS
*****************************************************************************
:91 FORMAT (1514)
:92 FORMAT (8F8.3)
:93 FORMAT (4E10.5)
:94 FORMAT (7A1)

:999 FORMAT (/*/)
:9999 FORMAT (H//)

:903 FORMAT (1H ,25x,'******************************************************',/ 1 1H ,25x, '*',/ 2 1H ,25x, 'CONSOLIDATION PROBLEM ',/ 3 1H ,25x,'*',/ 4 1H ,25x, 'STEP LOADING & SURCHARGE ',/ 5 1H ,25x, '******************************************************')
:904 FORMAT (1H ,10X,'THE PORE WATER Pressures ARE COMPUTED AT')
:905 FORMAT (1H ,10X,' YE/H ',5F10.3)
:906 FORMAT (1H ,10X,' XT/W ',5F10.3)
:907 FORMAT (1H0 ,10X,'THE PORE Pressures ARE INTERPOLATED AT')
:908 FORMAT (1H ,10X,' XE/W= ',5F10.3)
:909 FORMAT (1H ,10X,'ASSUMING COLLOCATION POLYNOMIALS OF DEGREE ')
:910 FORMAT (1H ,10X,I1,' BETWEEN THE LIMITS ',I8,3,' AND ',I8,3)
10X. 'HORIZONTAL PORE PRESSURES ARE COMPUTED AT' 
1H. 'TOTAL THICKNESS ', 'H=' , 'F8. 3. ', ' FEET '/
1H. 'reference for X-COORD W = ' , 'F8. 3. ', ' FEET '/
1H. 'LAYER INTERFACE IS ' , 'F8. 3. ', ' FT BELOW SURFACE '/
1H. 'LOWER/UPPER PERMEABILITY, RK= ' , 'F8. 3. /
1H. 'LOWER/UPPER COEF. OF CONSOLIDATION; RC= ' , 'F8. 3/
1H. 'LOWER/UPPER INITIAL VOID RATIO, REO= ' , 'F8. 3.
1H. 'DEGREE OF SATURATION IS S= ', 'F5. 3. /
1H. 'H-COEF(FT*FT/DAY)= ', 'F8. 3. /
1H. 'INITL PORE GAS PRESSURE IS PU= ', 'E12. 4. ', ' PSF'/
1H. 'INITIAL VOID RATIO = ', 'F6. 3. 
1H. 'INITIAL COEFF. OF COMPRRESSIBILITY IS AVD= ', 'F10.4', ' FT*FT /LB.' /
1H. 'THE COMPRESSION INDEX IS = ', 'E12. 4. /
1H. 'RECOMPRESSION INDEX/CC ROC'= ', 'F8. 3.)
1H. 'INITIAL EFFECTIVE P AND PRECOMPRESSION',/
1H. 'STRESSES PC AS USED IN THE COMPUTATIONS'/
1H. 'Y IN FT', 'F10. 4X, 'P IN PSF', '4X, 'PC IN PSF')
1H. 'THE INITIAL PERMEABILITIES ARE IN/',
1H. 'VERTICAL DIRN. KVD= ', 'E12. 4.', ' FT/DAY', /
1H. 'HORIZONTAL DIRN KHO= ', 'E12. 4.', ' FT/DAY' /
1H. 'ARE IN PPUT AT SPECIFIED EFFECTIVE STRESSES',/
1H. 3X, 'EFF-STRESS(PSF)', '2X, 'V-COEF(FT*FT/DAY)', /
2X, 'EFF-STRESS(PSF)', '2X, 'H-COEF(FT*FT/DAY)'/
1H. 'THE INITIAL PERMEABILITIES ARE IN/',
1H. 'VERT. PERM/VERT. IMPEDED PERM RKV= ', 'F5. 2. /
1H. 'THICKNESS OF IMPEDED LAYER, HI= ', 'F8. 3. ', 'FT' /
1H. 'THICKNESS OF THE DRAINAGE BLANKET YWM=' ', 'F6. 2.', ' FT', /
1H. 'THE TANGENT OF THE ANGLE ', /
1H. 'OF INTERNAL FRICTION TGPHI= ', 'F7. 4.', '/'
1H. 'MINP= ', 'I3. ', 'COOR POINTS XINP/YINP' /
1H. 'THE AVERAGE PORE PRESSURES, UAVER(I)' /
1H. 'THE CONSOLIDATION SETTLEMENTS, SETRC(I) AND THE',/
1H. 'TOTAL SETTLEMENTS, SETRT(I), AT XT(I) ARE' /
1H, 10X, 'XT FEET', 4X, 'UAVER (PSF)', 3X, 'SETRC', / 
' FEET', 3X, 'SRT Feet')
933 FORMAT (1H, 10X, F7.2, 4X, F12.2, 4X, F9.3, 3X, F9.3)
934 FORMAT (1H, 10X, 'COEFF. OF CONSOL. VERT FLOW IS CV= ', / 
1H, 10X, E12.4, ' FEET**2/DAY', / 
2H, 10X, 'COEFF. OF CONSOL. HDRI. FLOW IS CH= ', / 
3H, 10X, E12.4, ' FT**2/DAY')
935 FORMAT (1H, 10X, 'REFERENCE LOAD', / 
1H, 10X, '**********')
936 FORMAT (1H, 10X, 'THE NUMBER OF LIFTS IS NL= ', I3)
937 FORMAT (1H, 10X, 'SINCE ISP=1 TIMES OF LOAD APPLICATION', / 
1H, 10X, 'ARE INPUT TO BE', '///')
938 FORMAT (1H, 10X, 'TL(', I2, ')=', F6.0, ' DAYS')
939 FORMAT (1H, 10X, 'LOAD NO ', I3, ' APPLIED AT TL=', F6.0, ' DAYS', / 
1H, 10X, '**********')
940 FORMAT (1H, 10X, 'THE AVAILABLE CONSTRUCTION TIME IS TA= ', / 
1H, 10X, F6.0, ' DAYS. TA IS NOT NEEDED IF NL=1')
941 FORMAT (1H, 10X, 'THE REQUIRED SAFETY FACTOR IS FSI= ', F6.3, / 
1H, 10X, 'THE SPECIFIED PORTION OF SETLEMENT IS ', / 
1H, 10X, E3.3, ' "GUARD TO", FC 2')
942 FORMAT (1H, 10X, 'IF 95% OF THE AVE PORE PRESSURE', / 
1H, 10X, 'AT THE TIME OF APPLICATION OF THIS LOAD', / 
1H, 10X, 'HAVE DISSIPATED AT ', F6.4, '*', I2, ' POINTS XT', / 
1H, 10X, 'THIS LIFT IS ASSUMED TO BE THE LAST ONE')
943 FORMAT (1H, 10X, 'THE FACTOR OF SAFETY AT TIME T= ', F6.0, / 
1H, 10X, 'DAYS FOR LIFT ', I3, ' IS FS= ', F6.3, / 
2H, 10X, 'AS COMPARED TO THE REQ. FSI= ', F6.3)
944 FORMAT (1H, 10X, 'FS HAS BEEN OBTAINED FOR THE ARC WITH', / 
1H, 10X, 'X=', 'F8.2', ' Y=', 'F8.2', ' RADIUS=', 'F8.2, IN FT')
945 FORMAT (1H, 10X, 'FSI< FS= LIFT. TERMINATE THE PROGRAM')
946 FORMAT (1H, 10X, 'RESIDUAL PORE PRESSURES AS INPUT IN PSF', / 
1H, 10X, 'YE(I)/H=', 12X, 11F9.3)
947 FORMAT (1H, 10X, 'RESIDUAL PORE PRESSURES ARE IN PUT AS', / 
1H, 10X, 'X (FEET)', '3X, 'Y (FEET)', '3X, 'UC (PSF)')
948 FORMAT (1H, 10X, 'AT ', I3, ' OUT OF ', I3, ' POINTS XT', / 
1H, 10X, 'AN AVE DEGREE OF CONSOL. OF 95%', / 
2H, 10X, 'HAS COMPARED TO PORE PRESSURES AT THE TIME T= ', / 
3H, 10X, F6.0, ' DAYS OF LAST LOAD APPLICATION', / 
4H, 10X, '*** THIS LIFT NO. ', I3, ' . '
5H, 10X, 'IS THEREFORE CONSIDERED TO BE THE LAST ONE***', '///')
949 FORMAT (1H, 10X, 'EITHER THE FACTOR OF SAFETY, FS= ', F6.3, / 
1H, 10X, 'AND/OR THE SETTLEMENT, SETC(1)= ', F6.3, ' FEET', / 
2H, 10X, 'ARE LESS THAN SPECIFIED AT TIME T= ', F6.0, ' DAYS', / 
3H, 10X, 'WHICH IS GREATER THAN TA= ', F6.0, ' DAYS', / 
4H, 10X, '*** THIS LIFT NO. ', I3, ' IS, THEREFORE', / 
5H, 10X, 'CONSIDERED TO BE THE LAST ONE***', '///')
950 FORMAT (1H, 10X, 'THE CONSOL. PROCESS', / 
1H, 10X, '*************', '///', / 
2H, 10X, 'THE FOLLOWING INFORMATION IS OUT PUT', / 
3H, 10X, 'UAVE(X(1)), UAVE(X(2))........', /
144

1H, 10X, 'AVER DEGREES OF CONSOL. WITH RESPECT TO REF. LD

1H, 10X, 'SETC(X(1)), SETC(X(2)),

1H, 10X, 'CONSOL. SETTLEMENTS',

1H, 10X, 'SETI(X(1)), SETI(X(2)),

1H, 10X, 'IMMEDIATE SETTLEMENTS',

1H, 10X, 'SETT(X(1)), SETT(X(2)),

* 1H, 10X, 'CONSOLI. + IMMEDIATE SETTLEMENTS',

1H0.10X, 'LAST TWO LINE ARE ONLY OUT PUT',

1H, 10X, 'IF SOIL IS PARTIALLY SATURATED (B. NE. 1.)',

1H0.10X, 'THE POINTS X(I) IN FEET ARE AS FOLLOWS',

format (2X, 10F8.3)

format (/1H0, 10X, 'T= ', F6.0, /

1H0.10X, 'DAYS IS THE TIME OF LOAD APPLICATION', /

format (/1H0, 10X, 'AVE DEGREE OF CONSOL. AND SETTLEMENT', /

1H, 10X, 'CURVES FOR POINT X= ', F8.2, 'FEET FROM CENTER LINE', /

1H, 10X, 'INTERVAL BETWEEN 2 GRID LINES. EQ. 10%', /

1H, 10X, 'ABSCISSA NUMBERS GIVE THE TIME IN WKS.', /

1H, 10X, 'THE TOTAL SETTLE DUE TO REFERENCE LOAD IS', /

1H, 10X, E10. 3, 'FEET', /

1H0.10X, A1, ' -CURVE= AVE. DEGREE OF CONSOL.', /

7 1H, 10X, 'RELATIVE TO THE PORE PRESS DUE TO REF LOAD', /

8 1H0.10X, A1, ' -CURVE=CONSOL. SETTLL. IN % OF', /

9 1H, 10X, 'CURVE=CONSOL. SETTLEMENT')

format (1H, 10X, A1, ' -CURVE=IMMEDIATE SETTLEMENTS IN % OF', /

1 1H, 10X, 'THE REFERENCE SETTLEMENTS', /

2 1H, 10X, A1, ' -CURVE=TOTAL SETTLEMENTS IN % OF', /

3 1H, 10X, 'THE REFERENCE EMSETTLEMENT', /

format (1H0, 13, 76A1)

format (1H0, 2X, '0.0 0.1 0.2 0.3 0.4 0.5 0.6 0.7 0.8 ', /

0.9 1.0 1.1 1.2 1.3 1.4 1.5', '/)

format (1H0, 10X, 'THE FACTOR OF SAFETY AT TIME T= ', F6.0, /

1H, 10X, 'DAYS FOR LIFT', '13, 'WAS . GE.FS= ', F6.3, /

2 1H, 10X, 'AS COMPARED TO THE REQ. FS= ', F6.3)

format (1H, 20X, F8.3, 12X, F8.3, 12X, F8.3)

194 format (7A1)

format (1H, 10X, 'THE SHEAR STRENGTH CHARACTERISTICS OF', /

1 1H, 10X, 'THE SUB SOIL AS USED IN THE STABILITY ANALYSIS ARE', /

2 1H0, 15X, 'DEPTH (FT)', 5X, 'COHESION (PSF)', 5X, 'P-RATIO')

10000 END

subroutine LINT (X, Y, N1, M, XX, YY, N)
dimension X(1), Y(1), XX(1), YY(1)
C This subroutine interpolates the values of function Y(X)
C from the known YY(XX) by use of the linear interpolation.
C or extrapolation.
C
II=0
JJ=0
NN=M-N1+1
do 20 I=NN, M
   II=II+1
J=1
C Extrapolation is used if X(II).GT.XX(N)
if (X(II).LT.XX(N)) go to 31
Y(I)=YY(N)+(YY(N)-YY(N-1))*(X(II)-X(N))/(XX(N)-XX(N-1))
go to 20
31 if (X(II).GT.XX(1)) go to 32
C Extrapolation is used if X(II).LT.XX(1)
III=2
Y(I)=YY(1)-(YY(III)-YY(1))*(X(1)-X(II))/(XX(III)-XX(1))

20 continue
21 continue
}

- 144 -
goto 20
J=J+1
if (X(J).GT.XX(J)) go to 25
32 Interpolation is used if XX(I).LT.X(J).LT.XX(N)
35 If X(J) is very close to XX(J) then Y(I)=YY(J)
if (ABS(X(J)-XX(J)).GT.0.00000001) go to 28
Y(I)=YY(J)
go to 20
XY1=XX(J-1)
YY1=YY(J-1)
Y(I)=YY1+(X(J)-XY1)*(YY(J)-YY1)/(XX(J)-XY1)
20 continue
return
end
BEGIN SUBROUTINE INTEG
SUBROUTINE INTEG (ETA, XI, B, AR)
This subroutine computes the values of the stress integrals if the integration variable becomes larger than 12.
DIMENSION AR(7)
COMMON/ POAPI/ ALPHA(30), L
PI=3.14159265358979
ETAP=1.+ETA
ETAM=1.-ETA
DO 1 I=1,7
AR(I)=0.
DO 2 I=1,L
pq=1./1000000000000000.
AMX=ALPHA(I)-XI
IF (ABS(AMX).LT.pq) GO TO 3
AR(1)=AR(1)+PI-ATAN(ETAM/AMX)-ATAN(ETAP/AMX)
IF (AMX.LT.0.) AR(1)=AR(1)-2.*PI
APX=ALPHA(I)+XI
IF (ABS(APX).LT.pq) GO TO 4
AR(1)=AR(1)+PI-ATAN(ETAM/APX)-ATAN(ETAP/APX)
IF (APX.LT.0.) AR(1)=AR(1)-2.*PI
SIAMX=SIN(AMX*B)
COAMX=COS(AMX*B)
SIAPX=SIN(APX*B)
COAPX=COS(APX*B)
DPM=1./(ETAP*ETAP+AMX*AMX)
DPP=1./(ETAP*ETAP+APX*APX)
DMM=1./(ETAM*ETAM+AMX*AMX)
DMP=1./(ETAM*ETAM+APX*APX)
SM1=DPM*(ETAP*SIAMX+AMX*COAMX)
CM1=DPM*(ETAP*COAMX-AMX*SIAMX)
BP1=DPP*(ETAP*SIAPX+APX*COAPX)
CP1=DPP*(ETAP*COAPX-APX*SIAPX)
SM2=DMM*(ETAM*SIAMX+AMX*COAMX)
CM2=DMM*(ETAM*COAMX-AMX*SIAMX)
SP2-DMP*(ETAM*SIAPX+APX*COAPX)
CP2-DMP*(ETAM*COAPX-APX*SIAPX)

AR(2)=AR(2)+SM2+SP2
AR(3)=AR(3)+SM1+SP1
AR(4)=AR(4)+DPM*(ETAP*SM1+AMX*CM1)+DPP*(ETAP*SP1+APX*CP1)
AR(5)=AR(5)+CM2-CP2
AR(6)=AR(6)+CM1-CP1
AR(7)=AR(7)+DPM*(ETAP*CM1-AMX*SM1)+DPP*(APX*SP1-ETAP*CP1)

CONTINUE
EXAP=EXP(-ETAP*B)/2.
EXAM=EXP(-ETAM*B)/2.

AR(3)=AR(3)+SM1-SM1
AR(4)=AR(4)+DPM*ETAM-AMX*CM1+DPP*APX*CP1

AR(5)=AR(5)+SM1-SP1
AR(6)=AR(6)+CM1-CP1
AR(7)=AR(7)+DPM*ETAM-AMX*SM1+DPP*APX*CP1

CONTINUE
EXAP=EXP(-ETAP*B)/2.
EXAM=EXP(-ETAM*B)/2.

AR(1)=AR(1)/2.
AR(2)=EXAM*AR(2)*ETAM
AR(3)=EXAP*AR(3)
AR(4)=(EXAP*AR(4)+B*AR(3))*2.*ETA
AR(5)=EXAM*AR(5)*ETAM
AR(6)=EXAP*AR(6)
AR(7)=(EXAP*AR(7)+B*AR(6))*2.*ETA
AR(6)=AR(6)*ETAM

RETURN
END

SUBROUTINE COEF (UAVD, UAVE, OMEGA, PHI, LI, IL, OMED, PHID, NN)

This subroutine determines the soil parameters for the case that
they are variable.
If LI=3, the difference between the old and the new parameters
PHI and OMEGA are also computed and stored in PHID and OMED, resp.

DIMENSION UAVD(1), UAVE(1), OMEGA(1), PHI(1), OMED(1), PHID(1)
DIMENSION SVM(12), P(11), PC(11), PLOG(11), AA(1), BB(1)

COMMON/ SACSE/ ROC, ROCL, SVM, P, PC, PLOG, PO, PCO, IAV, IK, ISAT, AAV, AAH
COMMON/ SAC01/ AVO, KVO, KHO, EOPUS, PU, SKHM, SKVM, CCC, NNN, IDEF
COMMON/ SAC02/ PCV(10), CVIN(10), PCR(10), CHIN(10), ICV, KOUNT, HF

REAL K, KO, KV, KH, KVO, KHO

Statement functions for the computation of the coeff. of
permeability. CONK is used, when K is variable and AV is const.
VARK is used, when K and AV are variable. PSI computes the
c parameters to be returned to the calling program.

CONK(KO, SKM)=KO*EXP(-SKM*DISU)
VARK(KO, SKM)=KO*(PP/PQ)**(-1.*SKM)
PSI(AAA, K)=ALPHG*K*AAA/AV

The following parameters have been defined in program SAND and
are repeated here.
AAV=(1.+EO)/(GAMMAWATER*(DELTA Y)**2)
AAH=(1.+EO)/(GAMMAWATER*(DELTA H)**2)
SKVM=CC/SKV if IAV=1, SKVM=2.3026*AVO/SKV if IAV=0
SKHM=CC/SKH if IAV=1, SKHM=2.3026*AVO/SKH if IAV=0
AVO= initial coeff. of compressibility
KVO, KHO= initial coeff. in vert. and hori. dirns.
ALPHG = gas factor.
IF IAV=0, constant AV=AVD, IAV=1, variable AV
IK=0, const. K-s, IK=1, variable K-s.
NNN=no. of points with vert. and hori. flow.

IF KOUNT=0 if the subroutine is called the second or following time
IF KOUNT=1 if the subroutine is used for the first time
ICOEF=1 if IK=0 and IAV=0 or IAV=1
ICOEF=2 if IK=1 and IAV=0
ICOEF=3 if IK=1 and IAV=1
ICOEF=4 if K*(1.+EO)/(GAMMAWATER*AV*DELTA Y**2)=CVIN and
K*(1.+EO)/(GAMMAWATER *AV*DELTA H**2)=CHIN are
specified at ICV effective stresses PCV and PCH resp.

ISAT=0 full saturation, ISAT=1 partial saturation
PO= ave present overburden pressure
PCO= ave preconsol. pressure
DISU= UAVD-UAVE= ave dissipated pore pressure
CCC= 0.4343* compression index
EDPUS= EO*PU*(1.5*(1.-HC)), where PU=initial pore gas pressure,
5=initial degree of saturation, HC=Henry's const.
OMEGA(I)=(gas factor*coeff. of consol)/delta H**2
PHI(I)=(gas factor*coeff. of consol.)/delta Y**2
OMED(I)=difference between old and new omega(I)
PHID(I)=difference between old and new PHI(I)

IF (KOUNT.EQ.0) GO TO 20
ALPHG=1.
IF (IAV.EQ.1) PG=CCC/AVD

Determine OMEGA, PHI, OMED and PHID at NN points

Determine eff. stresses at the sum of the present overburden
stress PO and the dissipated pore pressures DISU. Determine
then the soil parameters as a function of PO+DISU

20 DO 5 I=1,NN
KK=ICOEF
DISU=UAVD(I)-UAVE(I)
PP=PO+DISU
AV=AVO
KV=KVO
KH=KHO
IF (KK.EQ.4) GOTO 6
IF (IAV.EQ.0) GO TO 6
IF (PP.GT.PG) GO TO 31
KK=1
GO TO 6
31 AV=CCC/PP
5 IF (ISAT.EQ.0) GO TO 60
AUX=AV
IF (PP.LT.PCO) AUX=AUX*ROC
ALPHG=1./((1.+EDPUS/(AUX*(PU+UAVE(I))**2))
60 IF (II.EQ.1) GO TO 7
IF (HF.EQ.0) RETURN

PARAMETERS FOR HORIZONTAL CASE

if (KK.eq.1) go to 11
if (KK.eq.2) goto 12
if (KK.eq.3) goto 13

2 KK=CONK(KHO, SKHM)
GO TO 11
13  KH=VARK(KHO, SKHM)
11  A=PSI(AAH, KH)
    GO TO 30
C C INTERPOLATE BETWEEN CHIN(I)
C 14  BB(1)= ALOG(PP)
    CALL LAGR(BB, AA, 1, 1, PCH, CHIN, ICV)
    A=ALPHG*AA(1)
30  IF (LI.EQ.3) OMEGA(NN+1-I)=OMEGA(NN+1-I)-A
    OMEGA(NN+1-I)=A
    GO TO 5
C C PARAMETERS FOR VERTICAL CASE
C 7    if (KK.eq.1) goto 1
    if (KK.eq.2) goto 2
    if (KK.eq.3) goto 3
    go to 4
2    KV=CONK(KVO, SKVM)
    GO TO 1
3    KV=VARK(KVO, SKVM)
1    A=PSI(AAV, KV)
    GO TO 40
C C INTERPOLATE BETWEEN CVIN(I)
C C 4    BB(1)= ALOG(PP)
    CALL LAGR(BB, AA, 1, 1, PCV, CVIN, ICV)
    A=ALPHG*AA(1)
40  IF (LI.EQ.3) PHID(I)=PHI(I)-A
    PHI(I)=A
5    CONTINUE
C RETURN
END
C BEGIN SUBROUTINE FUNCT
C SUBROUTINE FUNCT (THETA, ETA, K, SIGX, SIGY, TAU)
C This subroutine computes the values of the integrands for the
C argument theta
C COMMON/ POFUN/ G(516), ETHST(516)
C
TE=ETA*THETA
ETE=EXP(TE)
C=(ETHST(K)+1./ETHST(K))/2.
S=(ETHST(K)-1./ETHST(K))/2.
CTE=(ETE+1./ETE)/2.
STE=(ETE-1./ETE)/2.
C
D=G(K)/(C*C+THETA*THETA)
FA=C+THETA*S
FB=THETA*TE
FC=THETA*C
FD=TE*CTE
FE=TE*STE
EVALUATE THE INTEGRANDS

\[
\begin{align*}
\Sigma X &= -D \cdot (FA \cdot (CTE + FE) - FC \cdot (2 \cdot STE + FD)) \\
\Sigma Y &= -D \cdot (FA \cdot (CTE - FE) + FC \cdot FD) \\
\tau &= +D \cdot (-FA \cdot FD + FC \cdot (CTE + FE)) \\
\end{align*}
\]

IF (ETA. EQ. 1.) RETURN
AUX = 2. * Q(K) * CTE / ETHST(K)
\[
\begin{align*}
\Sigma X &= \Sigma X + AUX \\
\Sigma Y &= \Sigma Y + AUX \\
\end{align*}
\]
RETURN
END

END OF SUBROUTINE FUNCT

SUBROUTINE PORE (XINP, YINP, M, NST, CX, IX, CY, IY, U, ABAR, BBAR)

This subroutine computes the stresses within a compressible soil layer by use of elastic theory for plane strain conditions and a symmetric loading. Poisson's ratio is 0.5 and the underlying stratum is perfectly rough and rigid. Pore pressures are computed from a knowledge of the stresses and the pore pressure coeff. A and B called herein ABAR and BBAR.

DIMENSION XINP(1), YINP(1), CX(1), CY(1), U(1), SX(220)
DIMENSION SY(220), TA(220), GY(516), GT(516), SI(3)
DIMENSION SUM(3), R(3), T(3), DIF(3), AR(7), SUMS(3), ABSD(3)
COMMON/ SAPIOD/ IDUTP, W, H, GLOAD, CLOAD, NARC, NRAD
COMMON/ PAPI/ ALPHA(30), L
COMMON/ PDUN/ GST(516), ETHST(516)

If no load is specified (No. of input points M. LE. 1) set pore pressures equal to zero and return.

IF (M. GT. 1) GO TO 101
DO 100 I=1,220
100 U(I)=0.
RETURN

Approximate actual load by NST strips of const. thickness DST.

101 CALL APROX (XINP, YINP, M, NST, DST)
WRITE (IDUTP, 94) L
DD 10 I=1,L
WRITE (IDUTP, 95) I, ALPHA(I)
10 ALPHA (I) = ALPHA (I) / H

Define constants used in the stress computation.

DELTA=0.0001
DEL=0.001
DELT=DELTA
PI=3.14159265358979
IMAX=512
AINT=1./512.
KEND=IMAX+1
PMAX=NST
PMAX=ABS(PMAX*PI/2.)
JEND=12
FAC=2.*GLOAD*DST/PI
ZETA=W/H
IXIY=IX*IY
BRAN=2.

C Define constant for pore pressure determination.
C
CONST=0.8660254*(ABAR-1./3.)+1./2.
C
C SET STRESSES EQUAL ZERO
C
DO 21 I=1,IXIY
SX(I)=0.
SY(I)=0.
21 TA(I)=0.

C IER=0
C
C Initialize the numerical integration procedure
C Use Simpsons rule if XI.LT.BRAN
C Use Filons formula if XI.GE.BRAN
C Numerical integration is done between 0.0 and REAL(JEND)
C
DO 5 ISTEP=1,JEND
C
A=ISTEP-1
B=ISTEP
BA=1.

IF (ISTEP.EQ.2) DELT=DEL
C
C Compute factors repeatedly used in suroutine FUNCT
C
THETA=A-AINT
DO 2 K=1,KEND
THETA=THETA+AINT
ETHST(K)=EXP(THETA)
QST(K)=0.
C
DO 2 L=1,L
QI=ALPHA(I)
AT=QI*THETA
ABSAT=ABS(AT)
IF (ABSAT.LT.0.001) GO TO 3
QI=QI*SIN(ABSAT)/AT
3 IF (AT.LT.0.) QI=-QI
2 QST(K)=QST(K)+QI
C
C Integrate numerically between the limits A and B and store
C the results in one dimensional arrays SX,SY,TA
C
DO 5 J=1,IY
ETO=CY(J)
ETA=1.-ETO
DO 51 JGX=1,KEND
51 GX(JGX)=1.E15
C
C Evaluate the integrand at interval limits A and B
C
CALL FUNCT (A,ETA,1,GX(1),QY(1),QT(1))
CALL FUNCT (b, eta, kend, qx(kend), gy(kend), qt(kend))

DO 5 I=1, IX
LL=(I-1)*IY+J
XI=CI(I)*ZETA

C INITIALIZE INTEGRATION

DO 41 K=1, 3
SI(K)=0.
XIA=XI*A
XIB=XI*B
SIXIA=SIN(XIA)
SIXIB=SIN(XIB)
COXIA=COS(XIA)
COXIB=COS(XIB)
IF (XI.LT.BRAN) GO TO 1
DIF(1)=QX(KEND)*SIXIB-QX(1)*SIXIA
DIF(2)=QY(KEND)*SIXIB-QY(1)*SIXIA
DIF(3)=QT(1)*COXIA-QT(KEND)*COXIB
R(1)=QX(1)*COXIA+QY(KEND)*COXIB
R(2)=QY(1)*COXIA+QY(KEND)*COXIB
R(3)=QT(1)*SIXIA+QT(KEND)*SIXIB

C COMPUTE THE INTEGRAL BY INTERVAL HALVING

NHALF=1
N=2
AN=N
HH=BA/AN
XK=A-HH
XINC=2.*HH
DO 44 K=1, 3
T(K)=0.

Compute the values of the integrands if not yet computed and store them in qx(k), gy(k), qt(k)

IDEL=IMAX/NHALF
IT=-IMAX/N+1
DO 8 K=1, NHALF
XK=XK+XINC
IT=IT+IDEL
pq=100000000000000000
IF (QX(IT).EQ.pq) go to 55
go to 56

55 CALL FUNCT (xk, eta, it, qx(it), gy(it), qt(it))
56 XIX=XI*XK
COXIX=COS(XIX)
T(1)=T(1)+QX(IT)*COXIX
T(2)=T(2)+QY(IT)*COXIX
T(3)=T(3)+QT(IT)*SIN(XIX)
8 CONTINUE

IF (XI.GE.BRAN) GO TO 13
C SIMPSON RULE
C
HH=HH/3.
DO 46 K=1,3
46 SUM(K)=HH*(R(K)+4.*T(K))
  GO TO 4
C
C Filons formula
C
C
13 XIH=XI*HH
XXIH=XI*XIH
SIX=SIN(XIH)
COX=COS(XIH)
C=(SIX/XIH-COX)*4.
D=XIH+SIX*COX-2.*SIX*SIX/XIH
E=1.+COX*COX-2.*SIX*COX/XIH
DO 47 K=1,3
47 SUM(K)=(D*DIFF(K)+E*R(K)+C*T(K))/XXIH
C
C CHECK FOR ACCURACY
C THE END RESULT WAS FOUND TO BE LITTLE AFFECTED EVEN WHEN THIS
C ACCURACY WAS NOT REACHED.
C
4 IF (NHALF.EQ.1) GO TO 16
  SUMS(1)=SX(LL)+SUM(1)
  SUMS(2)=SY(LL)+SUM(2)
  SUMS(3)=TA(LL)+SUM(3)
C
C Accuracy as compared to the value of the integral between A and B
C
DO 48 K=1,3
  ABSD(K)=ABS(SUM(K)-SI(K))
  IF (ABSD(K).GT.ABS(DELT*SUM(K)))) GO TO 16
48 CONTINUE
C
IF (ISTEP.EQ.1) GO TO 17
C
C ACCURACY AS COMPARED TO THE MAXIMUM LOAD INTENSITY
C
DO 49 K=1,3
IF (ABS(K).GT.(DELTA*PMAX)) GO TO 16
49 CONTINUE
GO TO 17
C
16 NHALF=N
N=2*NHALF
IF (N.LE.IMAX) GO TO 19
IF (IER.NE.0) GO TO 191
IER=1
C
The Error Message at this point is suppressed to reduce the volume of the output.
C
WRITE ERROR MESSAGE, IF N.GT.IMAX
C
191 continue
GO TO 17
C
RENAME FOR NEXT CHECK OF ACCURACY
C
19 DO 50 K=1,3
SI(K)=SUM(K)
50 R(K)=R(K)+2.*T(K)
GO TO 6
C
17 SX(LL)=SUMS(1)
SY(LL)=SUMS(2)
TA(LL)=SUMS(3)
C
5 CONTINUE
C
Stresses in X- and Y- Dirn. not including factor FAC
C
DO 15 J=1,IY
ETO=CY(J)
ETA=1.-ETO
C
DO 15 I=1,IX
XI=CX(I)*ZETA
LL=(I-1)*IY+J
IF (ETA.NE.1.) GO TO 30
C
Stresses at the surface (ETA.EQ.1) not including factor FAC. Use modified formula for stresses at the surface.
C
SX(LL)=SX(LL)-SY(LL)
SY(LL)=0.
DO 32 IS=1,L
ABSAL=ABS(ALPHA(IS))
IF (((XI/ABSAL)-1.) .LT. 0.785398163397448*ALPHA(IS)/ABSAL
32 CONTINUE
SX(LL)=SX(LL)+SY(LL)
TA(LL)=0.
GO TO 31
C
Stresses below the surface (ETA.LT.1) not including factor FAC
CALL INTEG (ETA, XI, B, AR)
SX(LL)=SX(LL)-AR(1)+AR(2)-(3.-ETA)*AR(3)+AR(4)
SY(LL)=SY(LL)-AR(1)-AR(2)-(1.+ETA)*AR(3)-AR(4)
TA(LL)=TA(LL)+AR(5)+AR(6)-AR(7)

C Stresses in X- and Y-dirn and principal stresses
C Tension is positive
C
31
SSUM=(SX(LL)+SY(LL))/2.
SDIF=(SX(LL)-SY(LL))/2.
ROOT=SQRT(SDIF*SDIF+TA(LL)*TA(LL))
SX(LL)=FAC*SX(LL)
SY(LL)=FAC*SY(LL)
TA(LL)=FAC*TA(LL)
S1=FAC*(SSUM-ROOT)
S2=FAC*(SSUM+ROOT)

C Compute pore pressures for plane strain Conditions with
C poissons ratio of 0.5 using skempton's pore pressure parameters
C A and B, herein called ABAR and BBAR.
C CONST was earlier defined as CONST=0.5*(ABAR-1/3)+1/2.
C
U(LL)=-BBAR*(S2+(S1-S2)*CONST)

15
CONTINUE
WRITE (IOUTP,93)

C FORMAT STATEMENTS
C
93
94
FORMAT (//////)
FORMAT (1H,10X,'THE ACTUAL LOAD IS APPROXIMATED BY ',I3,/) 1
11X,' LOADS OF EQ INTNSTY WHICH EXTEND FROM X=0 TO ALPHA(I)',/
21H,10X,'IF ALPHA(I) IS NEGATIVE, THIS LOAD IS SUBTRACTED',/) 1

95
FORMAT (1H,15X,'ALPHA(','I2,' ) = ',F10.3,' FEET')

C RETURN
END

C END OF SUBROUTINE PORE
C BEGIN SUBROUTINE DISP
SUBROUTINE DISP (U,LI,OMEGA,PHI,T,UAVE,LIFT,MYE,IEND,XT,SY)

C This subroutine computes the pore pressures at time T by
C treating the consolidation equation as an eigenvalue
C problem.
C U
C contains the additional pore pressures for the new load
C when this subroutine is called.
C LI Load identifier.
C LI=1 Determination of vectors A and B for the load addition.
C LI=2 Determination of pore pressures due to the stepwise
C const. loads.
C LI=3 Determination of vectors A and B for times between
C load application if the soil parameters are variable.
C LI=5 First LIFT, first use of DISP.
C ISP=1 Compute and print the pore pressures at all points of
ISP=0 Compute only the average pore pressures at different depths at all IEND points.

IVAR identifier for soil parameters

IVAR=0 Constant soil parameters

IVAR=1 Variable soil parameters

PHI Vector depending on the soil parameters for vert. flow.

OMEGA vector depending on the soil parameters for H-flow.

T Time

LIFT no. of vectors A and B

M Program numbering system, No. of points in Vert. Dirn.

N Program numbering system, No. of points in H-dirn.

Storage reservations are made for 40 horizontal points and 6 step loads.

DIMENSION U(1), PHI(1), OMEGA(1), UAVE(1), XT(1), SV(1)

DIMENSION EIGV(10), EIGX(40), AUX(160), XV(100), XVI(100)

DIMENSION XH(1600), XHI(1600), FH(500), F(200), A(1200), B(1200)

DIMENSION VJ(11), RJ(40), RJJC20), G(280), UBAV(240), UBE(20)

DIMENSION W(100)

EQUIVALENCE (G(1), A(501))

COMMON/ SAPOD/ IDOUTP, W, H, GLOAD, CLOAD, NARC, NRAD

COMMON/ SADD/ LAYER, IBCV, MHE, M, N, IDC, NDR, ISUM, XET(41)

COMMON/ SADI2/ FIMPV, RC, RK, C, RO, RE, TA, ISP, IVAR

LIM=LI

IF (L.LT.5) GO TO 2

IF (LAYER.LT.3) LAYER=2

Call MODAL for determination of eigen values, modal matrix and inverse of the modal matrix for the case of vertical and horizontal flow

CALL MODAL (LAYER, IBCV, M, FIMPV, RC, RK, O., H, EIGV, XV, XVI, AUX)

PAGE 60

IF (IDC.EQ.1) GO TO 1

CALL MODAL (1, 4, N, FIMPV, RC, RK, RD, RE, EIGX, XH, XHI, AUX)

1 LIM=1

Determine the diagonal matrices F and FH

2 CALL EFGEN (PHI, T, EIGV, IVAR, IEND, M, F, LI)

IF (IDC.EQ.1) GO TO 3

CALL EFGEN (OMEGA, T, EIGX, IVAR, 1, N, FH, LI)

Branch depending on the value of LIM

3 GO TO (4, 5, 6), LIM

4 LIM=1

Determine vectors A and B for the LIFT-th load addition

Determination of vector B for the LIFT-th load addition

IB=(LIFT-1)*M*IEND
IE=0
DO 10 K=1, IEND
DO 11 I=1, M
IA=(K-1)*MYE+1
II=I-M
IB=IB+1
IE=IE+1
B(IB)=0.
DO 12 J=1, M
II=II+M
IA=IA+1
12 B(IB)=B(IB)+XVI(II)*U(IA)
   DO 340 iief=1, iief
340 B(IB)=B(IB)/F(IE)
11 CONTINUE

C IF (IDC.EQ.1) GO TO 13
C DETERMINE VECTOR A--HORIZONTAL CASE
C
II=0
LIN=(LIFT-1)*N+1
LIN2=LIFT*N
DO 121 I=1, IEND
UBB(I)=0.
DO 120 J=1, MYE
II=II+1
UBB(I)=UBB(I)+U(II)*SV(J)
120 CONTINUE
121 CONTINUE
CALL LINT (XET, UBAV, N, LIN2, XT, UBB, IEND)

C To avoid numerical instability put UBAV(I)=0.1 if it is zero.
C
DO 1210 I=LIN, LIN2
   IF (ABS(UBAV(I)).LT.0.1) UBAV(I)=0.1
1210 CONTINUE
IS=(LIFT-1)*N
DO 14 I=1, N
IE=0
II=I-N
IS=IS+1
A(IS)=0.
IA=LIN2+1
DO 15 J=1, N
II=II+N
IA=IA-1
IE=IE+1
15 A(IS)=A(IS)+XHI(II)*UBAV(IA)
   DO 341 iief=1, iief
341 A(IS)=A(IS)/FH(I)
14 CONTINUE

C IF (ISP.EQ.0) RETURN
C LIM=2
C DETERMINE THE PORE Pressures AT TIME T AT XT(J), J=1, IEND
C
NM=M
III=MYE
DO 50 I=1, ISUM
50
CONSIDER INFLUENCE OF LIFT LOADS

DO 53 K=1,LIFT
   IB=ID+M*ID+1
   IE=(J-1)*N+1
   CALL MAMULP (XV, F, B, VJ, M, IB, ID)

BRANCH IF VERT FLOW ONLY

IF (IDC.EQ.1) GO TO 54
   IB=1+N*(K-1)
   goto 54

CALL MAMULP (XH, FH, A, RJ, N, IB, 1)
   do 75 IR=1,N
       RJ(IR)=RJ(IR)/UBAV(K*N+1-IR)
   continue

   DO 650 IR=1,N/2
       HOLD=RJ(IR)
       RJ(IR)=RJ(N-IR+1)
       RJ(N-IR+1)=HOLD

   continue

Determine the average pore pressures at M+1 points in vertical direction at XT(J). Include the free drainage at the upper boundary. The result after LIFT cycles of loop 53 is returned to the calling program. The drainage at the lower boundary is considered outside loop 53.

CALL LAGR (XT, RJJ, IEND, 1, XET, RJ, N)
call LINT (XT, RJJ, IEND, IEND, XET, RJ, N)

54   II=(J-1)*MYE+1
   U(II)=0.
   DO 57 I=1,M
   II=II+1
   IF (IDC.EQ.1) GO TO 55
   RJAVE=RJ(J)
   if (RJAVE.LT.0.) RJAVE=0.0
   GO TO 57

55   RJAVE=1.
57   U(II)=U(II)+VJ(I)*RJAVE

CONTINUE

IF (IBCV.EQ.3) GO TO 58
   II=J*MYE
   U(II)=FIMPV*U(II-1)

58 IF (ISP.EQ.0) GO TO 59
   IF (IDC.EQ.1) GO TO 60

Output the pore pressures at XT(J) for the case of Vert+ Hor flow

WRITE (IOUTP,94) T,XT(J)
GO TO 62

C Output the pore pressures at XT(J) for the case of Vert flow only

60  WRITE (IDOUTP,91) T,XT(J)
61  II=(J-1)*MYE
   DO 61 I=1,MYE
   II=II+1
62  WRITE (IDOUTP,92) U(I)
   WRITE (IDOUTP,93)
   GO TO 59

C Compute the ave pore pressures at XT(J)

59  II=(J-1)*MYE
   UAVE(J)=0.
   DO 63 I=1,MYE
   II=II+1
63  UAVE(J)=UAVE(J)+U(I)*SV(I)
   CONTINUE
   RETURN

C Determine vectors A and B for the case of variable soil parameters and times between load applications LIM=3

C ***********************

C Determine vector B

6  II=IEND*M
   IB=0
   DO 20 K=1,LIFT
   DO 20 I=1,II
   IB=IB+1
   do 20 iief=1,ief
20  B(IB)=B(IB)*F(I)
   IF (IDC.EQ.1) RETURN

C DETERMINE VECTOR A

II=N
   IA=0
   DO 22 K=1,LIFT
   DO 22 I=1,II
   IA=IA+1
   do 22 iief=1,ief
22  A(IA)=A(IA)*FH(I)
   RETURN

C FORMAT STATEMENTS

91  FORMAT (///,1H,10X,'T= ',F6.0,' DAYS,X/W=',F8.3,/' PORE PRESSURES IN PSF-VERTICAL FLOW ONLY',//)
92  FORMAT (1H,10X,11F10.3)
93  FORMAT (///)
94  FORMAT (///,1H,10X,'T= ',F6.0,' DAYS X/W=',F8.3,/' PORE PRESSURES IN PSF DUE TO VERT + HORI FLOW',//)
SUBROUTINE SETL (U, SETTL, IEND, KKK, MYE, F, FUP, FLO, KIAV)

This subroutine computes consol. (F=1) or total (F.GT.1) settlements using constant (KIAV=1+IAV=1) or variable KIAV=2 soil parameters.

U Vector of dissipated pore pressures with IEND*MYE elements.

SVM Modified mathematical molecule for Simpsons or trapezoidal formula (considers case of stratified soil if LAYER.GT.2)

SETTL Vector of computed settlements with IEND elements

IEND No. of settlements

KKK No. of distinct pore pressures in the upper layer

MYE No. of distinct pore pressures in both layers

F Consol. settlements are computed if F=1.0

Total settlements are computed if F=1.0/B where B= Skempton pore pressure coeff.

FUP Parameter for the upper layer which incorporates the soil coeff.

FLO Corresponding parameter for the lower layer.

DIMENSION U(1), SETTL(I), SVM(12), P(11), PC(11), PLOG(11)

COMMON/ SACSE/ ROC, ROCL, SVM, P, PC, PLOG, PO, PCO, IAV, IK, ISAT, AAV, AAR

ISE=ISE+1

format ("NO. of times entered SETL = ", I5)
write (IOUTP, 901) ISE
R=ROC
A=FUP
II=-MYE
JST=1
JND=KKK
JSS=0

DO 1 I=1, IEND
1 SETTL(I)=0.

DO 2 I=1, IEND
2 II=II+MYE
IU=II
S=0.
JS=JSS

DO 3 J=JST, JND
3 IU=IU+1
JS=JS+1
GO TO (4, 5), KIAV

Constant soil parameters IAV=0 KIAV=1

4 S=S+F*SVM(JS)*U(IU)
GO TO 3
Variable soil parameters IAV=1 KIAV=2

if the current effective stress at some point becomes negative then it is put as P(J) at that point.
i.e no swelling is considered.

if (PP.LT.P(J)) PP=P(J)
IF(PP.GT.PC(J)) GO TO 7
S=S+R*SV(J)*ALOG(PP/P(J))

GO TO 3
7 S=S+SV(J)*ALOG(PP/PC(J))+R*PLG(J))
3 CONTINUE
SETTL(I)=SETTL(I)+A*S
2 CONTINUE

Consol. of the lower layer

IF (JN.EQ.MYE) RETURN
A=FLO
R=ROCL
II=KKK-MYE-1
JST=KKK
JND=MYE
JSS=KKK
GO TO 6
END

SUBROUTINE MAMULP (A, D, B, C, N, IS, II)

This subroutine performs the matrix multiplication
(General matrix A)*(Diagonal matrix D)*(Vector B) = (Vector C)
All matrices are stored one dimensionally with A having N*N
elements and D, B, C each having N elements. The first element
of B and D are B(IS) and D(II)
The formula for the I-th element of C is
C(I)=SUM(K=1,N) of A(I+(K-1)*N)*B(IS-1+K)*D(XI+K-1)

DIMENSION A(1), B(1), C(1), D(1)

ieff=64
do 1 I=1,N
IA=N*N+I
IB=IS+N
ID=II+N
C(I)=0.
DO 1 K=1,N
IB=IB-1
ID=ID-1
IA=IA-N
cc=A(IA)*B(IB)
do 20 iieff=1,ieff
cc=D(ID)*cc
C(I) = C(I) + CC
I CONTINUE
RETURN
END
C END OF SUBROUTINE MAMULP
C SUBROUTINES BEGIN HERE.
C
SUBROUTINE OVERFLO (J)
C
J=1
RETURN
END

SUBROUTINE HDIST (UB, XT, IEND, ICV, CHIN, DXSG, AAH, MHE, W, XET, IPDR, HF *
*, MYE, POR)
C Begin subroutine computes the horizontal distance from the center line to the point
C where the pore pressure is 0.1% of the maximum pore pressure under the embankment. This is taken to be the Hori. drainage distance. This subroutine will be active only once.
C dimension UB(1), XT(1), CHIN(1), XET(1)
if (HF. eq. 0.) goto 100
if (IPDR. eq. 1) goto 100
C Find maximum pore pressure
50 do 1 = 1, IEND
if (umax. lt. UB((i-1)*MYE+1)) umax = UB((i-1)*MYE+1)
continue
umin = umax * 0.01
i = 1
60 if (UB((i-1)*MYE+1). lt. umin) goto 65
i = i + 1
if (i. lt. (IEND+1)) goto 60
POR = 1. + (1. - XT(IEND-1)/XT(IEND)) * (UB((IEND-1)*MYE+1) - umin) / (UB((IEND-2)*MYE+1) - UB((IEND-1)*MYE+1))
goto 70
65 POR = (XT(i-1) + (XT(1) - XT(i-1)) * (UB((i-2)*MYE+1) - umin) / (UB((i-2)*MYE+1) - UB((i-1)*MYE+1))) / XT(IEND)
70 IPOR = 1
C Redefine horizontal grid points and the related parameters
C using new horizontal drainage distance.
C do 529 I = 1, ICV
CHIN(I) = CHIN(I) * DXSG
529 continue
AAH = AAH * DXSG
AI = MHE - 1.
DHX = POR * XT(IEND) * W/AI
do 25 I = 1, MHE - 1
XET(I) = POR * (I-1) * XT(IEND) / AI
25 continue
DXSG = DHX * DHX
AAH = AAH / DXSG
do 530 i = 1, ICV
CHIN(i) = CHIN(i) / DXSG
530 continue
write (IATP, 778) POR
778 format ('NEW POR=', F10.5)
100 return
SUBROUTINE APROX (X, Y, MN, N, D)

C This subroutine approximates the actual load by N strips of
C constant thickness D

DIMENSION X(1), Y(1), XA(25), YA(25)
COMMON/ POAPI/ ALPHA(30), L

C DETERMINE MAX VALUE OF Y(1)

M=MN
YMAX=Y(1)
DO 1 I=2, M
   IF (Y(I) .GT. YMAX) YMAX=Y(I)
1 CONTINUE

C Initiate first step starting with X(M) and Y(M)

AN=N
D=YMAX/AN
L=1
H=Y(M)
XX=X(M)
XA(1)=0.

C Compute the portion between two horizontal lines with distance D

2 YA(1)=H
   K=2
   XA(K)=XX
   YA(K)=H
   H=H+D

C If statement because of possible truncation error

3 IF (ABS(H-YMAX) .LT. 0.001) H=YMAX
   MM=M-1
   IF (MM.EQ.0) GO TO 4
   XX=(X(M)-X(MM))*(H-Y(MM))
C IF (ABS(Y(M)-Y(MM)) .LT. 0.01) J=1
   IF (ABS(Y(M)-Y(MM)) .LT. 0.01) CALL OVERFLO (J)
   IF (ABS(Y(M)-Y(MM)) .LT. 0.01) GO TO 61
   XX=XX/(Y(M)-Y(MM))

C If the denominator approaches zero, J is set equal to 1

61 IF (J .NE. 1) GO TO 5
   K=K+1
   J=0
   XA(K)=X(MM)
   YA(K)=Y(MM)
   M=MM
   GO TO 3
5 XX=XX+X(MM)
   IF (ABS(XX-X(MM)) .LT. 0.001) GO TO 8
   IF (XX.LT.X(MM)) GO TO 6
   MM=M

end
Determine width of constant load equivalent to portion between two horizontal lines with distance D.

ALPHA is negative if this portion is to be subtracted.

\[
\text{ALPHA}(L) = A/(2 \times \text{ABS}(D))
\]

IF (L.EQ.N) GO TO 11
IF (ALPHA(L).EQ.0.) L=L-1
IF (MM.EQ.0) GO TO 12
L=L+1
GO TO 2

IF (YMAX.LE.Y(1)) GO TO 12

D=-1.*D
GO TO 13

D=ABS(D)
RETURN
END

SUBROUTINE DETFS
SUBROUTINE DETFS (XC, YC, R, XINP, YINP, MINP, MX, MYE, SU, FS)

This subroutine determines the factor of safety against failure along a circular arc by taking the ratio of the resisting and driving moments about the center of the arc. Shear strengths along that part of the arc, which passes through the subsoil, are obtained by interpolating between the elements of vector SU. This is an analysis in terms of TOTAL STRESSES.

DIMENSION XINP(1), YINP(1), SU(1), XS(22), YS(22), X(2)
DIMENSION WWW(2), XAUX(10), YAUX(10), SINUS(2), COSIN(2)

REAL MD, MR

COMMON/ INDET/ RH0U9), TAUU9), PSI19)
COMMON/ SAPOD/ IOUTP, W, H, GLOAD, CLOAD, NARC, NRAD
COMMON/ SADET/ XSTAB(51), YSTAB(11), DX, DY, YWM, TGPHI

STATEMENT FUNCTIONS

FUNA(A, B) = (XC+A*AA)/B
FUNB(B) = AB*AB+(RX-AA*AA)/B
FUNC(A, B, C) = A+B*(C-A)

The parameters have the following significance
C XC, YC coord of the center of the arc.
C R radius of the arc
C XINP, YINP Coord of the points describing the X-section of
C the embankment.
C MINP No. of points XINP/YINP
C MX No. of equally spaced points in X-dirm.
C MYE No. of equally spaced points in Y-dirm.
C SU Vector of shear strengths with MX*MYE elements
C FS Factor of safety
C RHO(I) Slope of the line connecting XINP(I)/YINP(I) and
C XINP(I+1)/YINP(I+1)
C PSI(I) YINP value of the above line for XINP=0.
C TAU(I) TAU(I)=1.+RHO(I)*RHO(I)
C GLOAD unit weight of the embankment soil
C CLOAD Shear strength of the embankment soil
C XSTAB MX equally spaced points in X-dirm.
C YSTAB MYE equally spaced points in Y-dirm.
C DX, DY Intervals in X- and Y-dirm.
C NARC One-half no. of subarcs within subsoil
C
C ANARC=2*NARC
MXM=MX-2
RR=R*R
XX=XC*XC
RX=RR-XX
YY=YC*YC
LAST=0
C
C POINTS OF INTERSECTION BETWEEN ARC AND SURFACE
C
AA=SQRT(RR-YY)

C
XS(1)=XC-AA
XG=XC+AA
IF (XG.GE.XINP(MINP)) LAST=1
C
C POINT OF INTERSECTION BETWEEN ARC AND EMBANKMENT SURFACE
C
I=0
J=2
I=I+1
AA=YC-PSI(I)
AB=FUNA(RHO(I), TAU(I))
AA=FUNB(TAU(I))
IF (AA.LT.0.) GO TO 1
AA=SQRT(AA)
XT=AB-AA
IF (XT.GE.XINP(I+1)) GO TO 1
XS(2)=XT
YS(2)=XT*RHO(I)+PSI(I)
C
C Resisting moment MR due to the arc between XS(1)/YS(1)=0. and
C XS(2)/YS(2) within the embankment. Driving moment MD due to
C the segment between XS(1)/0. and XS(2)/YS(2).
C
BETA1=0.5*ASIN(YC/R)
BETA2=0.5*ASIN((YC-YS(2))/R)
MR=2.*RR*CLOAD*(BETA1-BETA2)
A = XS(1) - XS(2)
A = SQRT(A*A + YS(2)*YS(2))
IF (LAST.EQ.0) GO TO 2

C THE POINTS XS/YS ARE EQUAL TO THE POINTS XINP/YINP

I = I + 1
DO 3 K = I, MINP
J = J + 1
XS(J) = XINP(K)
YS(J) = YINP(K)
31 LAST = J
GO TO 7

C Determine the second point of intersection between the arc and the embankment surface. Store all points in XS/YS which lie between this point and the point XS(2)/YS(2).

2 XT = AB + AA
IF (XT.LE.XINP(I+1)) GO TO 4
I = I + 1
J = J + 1
XS(J) = XINP(I)
YS(J) = YINP(I)
IF (I.EQ.MINP) GO TO 31
IF (XINP(I+1).GT.XG) GO TO 6
IF (YINP(I+1).LT.YC) GO TO 5
6 AA = YC - PSI(I)
AB = FUNA(RHO(I), TAU(I))
AA = FUNB(TAU(I))
AA = SQRT(AA)
GO TO 2

4 J = J + 1
XS(J) = XT
YS(J) = XT*RHO(I) + PSI(I)

Resisting and driving moments due to the arc and the segment between the points XS(J)/YS(J) and XG/O.

BETA2 = 0.5*ASIN((YC-YS(J))/R)
LAST = J + 1
XS(LAST) = XG
YS(LAST) = 0.
MR = MR + 2.*RR*CLOAD*(BETA1 - BETA2)
A = XG - XS(J)
A = SQRT(A*A + YS(J)*YS(J))

DRIVING MOMENTS DUE TO TRIANGLES WITH ONE APEX AT XS(1)/0.

7 XX = 3.*XC - XS(1)
DO 8 I = 3, LAST
J = I - 1
A = XS(I)*(YS(I) - YS(J)) - XS(J)*YS(I) + XS(I)*YS(J)
MD = MD + (XX - XS(J) - XS(I))*A
8 CONTINUE

RESISTING MOMENT DUE TO THE PART OF THE ARC WHICH PASSES THROUGH
C TH SUBSOIL
C
IF (H.EQ.0.) GO TO 100
IF (MY.EQ.1) GO TO 20
C
C Resisting moment along subarcs in the subsoil. The shear strengths along the 2*NARC subarcs are assumed const and obtained by linear interpolation between the appropriate values of SU.
C
DARC=(3.1415927-4.*BETA1)/ANARC
RARC=RR*DARC
BETA=2.*BETA1-DARC/2.
DO 9 L=1,NARC
BETA=BETA+DARC
A=R*COS(BETA)
X(1)=XC-A
X(2)=XC+A
A=R*SIN(BETA)-YC
AJ=A/DY+1.
J=AJ
FY=(A-YSTAB(J))/DY
C
DO 10 K=1,2
AII=X(K)/DX
I=AII
IF (I.LE.MXM) GO TO 11
C
C Midpoint of the subarc lies outside the domain for which SU-s are specified. Interpolation is done in Y-dirn only between the values SU(MX*(MYE-1)+1) through SU (MX*MYE).
C
IJ=J+(MX-1)*MYE
JJ=IJ+1
AA=SU(IJ)
AB=SU(JJ)
GO TO 12
C
C Interpolation for the midpoint of the subarc. Two linear interpolation are performed in X-dirn. One linear interpolation is performed in Y-dirn between the values obtained.
C
11 IJ=J+I*MYE
JJ=IJ+MYE
I=I+1
FX=(X(K)-XSTAB(I))/DX
AA=FUNC(SU(IJ),FX,SU(JJ))
IJ=IJ+1
JJ=JJ+1
AB=FUNC(SU(IJ),FX,SU(JJ))
12 MR=MR+RARC*FUNC(AA,FY,AB)
10 CONTINUE
DM=MD*GLOAD/6.
9 CONTINUE
GO TO 100
20 RARC=RR*(3.1415927-4.*BETA1)
MR=MR+SU(1)*RARC
C
C Factor of safety.
C
MD = MD*GLOAD/6.

C IF THERE IS AN OVERFLOW CONDITION, J IS SET. EQ. 1 AND
C FS IS SET. EQ. 1.0E50

JJJ = 0
IF (MD.LT.0.00000000000000001) CALL OVERFLO (JJJ)
IF (MD.LT.0.00000000000000001) GO TO 111
FS = MR/MD

111 IF (JJJ.NE.1) GO TO 40
FS = 100
JJJ = 0
RETURN

C Determination of the resisting moment due to friction:

40 IF (YWM.EQ.0.) RETURN
IF (FS.LT.0.0001) FS = 1.
FAC = TQPHI/FS
RMR = MR
KOUNT = 1
XAux(1) = XS(1)
YAux(1) = 0.
AB = YC - YWM
AB = SQRT(RR - AB*AR)
XAux(2) = XC - AB
YAux(2) = YWM
IF (XAux(2).GE.XS(2)) GO TO 41
XAux(2) = XS(2)
YAux(2) = YS(2)

41 BETA2 = 0.5*ASIN((YC-YAux(2))/R)
XAux(3) = XAux(2)
K = 3
I = 1
43 I = I + 1
42 IF (XAux(K)-XINP(I)) 44, 45, 43
45 YAux(K) = YINP(I)
GO TO 46
44 YAux(K) = RHO(I-1)*XAux(K)+PSI(I-1)
46 K = K + 1
IF (XAux(1).LE.XINP(I)) GO TO 47
XAux(K) = XINP(I)
YAux(K) = YINP(I)
I = I + 1
GO TO 46
47 XAux(K) = XAux(1)
YAux(K) = RHO(I-1)*XAux(1)+PSI(I-1)
XAux(K+1) = XAux(1)
YAux(K+1) = YAux(1)
WW = 0.
DO 48 J = 1, K
L = J + 1
WW = WW - XAux(J)*YAux(L)+YAux(J)*XAux(L)
48 CONTINUE
WWW(KOUNT) = WW*GLOAD/2.
AA = XAux(1) - XAux(2)
BB = YAux(2) - YAux(1)
CC = SQRT(AA*AA+BB*BB)
SINUS(KOUNT) = BB/CC
COSIN(KOUNT) = AA/CC
RMR = RMR - 2. * RR * CLOAD * (BETA1 - BETA2)

IF (KOUNT EQ. 2) GO TO 49
IF (XS(LAST) EQ. XINP(MINP)) GO TO 49
KOUNT = 2
XAUX(1) = XC + AB
YAUX(1) = YWM

IF (XAUX(1) LE. XS(LAST - 1)) GO TO 50
XAUX(1) = XS(LAST - 1)
YAUX(1) = YS(LAST - 1)

BETA2 = 0.5 * ASIN((YC - YAUX(1)) / R)
XAUX(2) = XS(LAST)
YAUX(2) = 0.
XAUX(3) = XAUX(2)
K = 3
GO TO 42

C ITERATION FOR THE CORRECT FACTOR SAFETY
C
49 MR = RMR
DO 51 I = 1, KOUNT
MR = MR + FAC * WWW(KOUNT) * R /
(COSIN(KOUNT) + FAC * SINUS(KOUNT))
51 CONTINUE
FSOLD = FS
FS = MR / MD
IF (ABS(FS - FSOLD). LT. 0.001) RETURN
FAC = TQPHI / FS
GO TO 49
END

C END OF SUBROUTINE DETFS
C
C BEGIN SUBROUTINE GAIN
C
SUBROUTINE GAIN (UA, R, SU, MYE, MXT, MXE, MX, NIM, CO, CP, III)
C
This subroutine determines the shear strengths at MX*MYE points
XE/YE from a knowledge of the initial shear strengths CO and
the C/PBar=CP-ratios
C
UA Dissipated pore pressures at XT/YE
C R Auxiliary matrix for the computation of the dissipated pore
pressures at XT/YE from those at XT/YE
C SU Shear strengths at XE/YE. SU(XE/YE) is equal to the initial
shear strength CO(YE) plus the product of the dissipated
pore pressure at XE/YE and the C/PBar-ratio CP(YE)
C MYE No. of points YE in vertical direction.
C NIM No. of intervals in horizontal Dirn.
C MXT NIM numbers of points XT in each interval.
C MXE NIM no. of points in XE in each interval
C MX Total no. of points XE
C CP Vector of MYE C/PBAR-ratios
C CO Vector of MYE initial shear strengths
C III Index for the identification of the following cases—
C If III=1, all elements of UA are assumed to be zero.
C If III=0, some or all elements of UA are not equal to zero
C
DIMENSION UA(1), R(1), SU(1), MXT(1), MXE(1), CO(1), CP(1)

K = 0
IF (III EQ. 0) GO TO 1
C
C ALL ELEMENTS OF UA ARE EQUAL TO ZERO
The shear strength consists of the initial strength plus some gain due to dissipated pore pressures.

```fortran
C
DO 2 I=1, MX
DO 2 J=1, MYE
K=K+1
SU(K)=CO(J)
2 CONTINUE
RETURN
C
IUBE=0
IUND=0
IRND=0
DO 3 JJ=1, NIM
IUBS=IUBE+1
IUBE=IUBE+MYE*MXT(JJ)
IUST=IUND+1
IUND=IUND+MYE*MXE(JJ)
IRST=IRND+1
IRND=IRND+MXT(JJ)*MXE(JJ)
CALL MPRD (UA, R, SU, MYE, MXT(JJ), MXE(JJ), IUBS, IRST, IUST)
3 CONTINUE
DO 4 I=1, MX
DO 4 J=1, MYE
K=K+1
SU(K)=CO(J)+SU(K)*CP(J)
4 CONTINUE
RETURN
END
```

END OF SUBROUTINE GAIN

BEGIN SUBROUTINE GENER

SUBROUTINE GENER (P,F,X,N)
This subroutine generates the \( N+1 \) Coeff. of the characteristic equation of the tridiagonal matrix \( P \). The coeff. are stored in vector \( A \). \( P \) must be supplied as a one-dimensional array with \( 2*N \) elements. An auxiliary vector \( F \) with \((N+1)*(N+4)/2-2\) elements is used for computation. Vector \( X \) contains the \( N \) roots of the characteristic equation.

\[
\text{DIMENSION } P(1), A(25), F(2), X(1)
\]

\[
\text{IC}=0, \quad F(1)=0, \quad F(2)=1.0, \quad \text{IF}=2, \quad \text{NF}=N
\]

\[
\text{COMPUTE } F(3) \text{ THRU } F(N+2)
\]

\[
\text{DO } 1 \text{ I}=1, \text{NF}
\]
\[
\text{IF } =\text{IF}+1
\]
\[
\text{IP}=2*I
\]
\[
F(\text{IF})=P(\text{IP}-1)*F(\text{IF}-2)+P(\text{IP})*F(\text{IF}-1)
\]
\[
1 \text{ CONTINUE}
\]

\[
\text{IC}=\text{IC}+1
\]
\[
A(\text{IC})=F(\text{IF})
\]
\[
\text{IF}(\text{IC.EQ.N}) \text{ GO TO } 3
\]
\[
\text{NF}=\text{NF}-1
\]

\[
\text{COMPUTE } F(N+3) \text{ THRU } F((N+1)*(N+4)/2-2)
\]

\[
F(\text{IF}+1)=0.
\]
\[
\text{IF}=\text{IF}+2
\]
\[
\text{II}=\text{IF}-\text{NF}-3
\]
\[
\text{IP}=2*IC
\]
\[
F(\text{IF})=1.
\]
\[
\text{DO } 2 \text{ I}=1, \text{NF}
\]
\[
\text{IF}=\text{IF}+1
\]
\[
\text{II}=\text{II}+1
\]
\[
\text{IP}=\text{IP}+2
\]
\[
F(\text{IF})=P(\text{IP}-1)*F(\text{IF}-2)+P(\text{IP})*F(\text{IF}-1)+F(\text{II})
\]
\[
2 \text{ CONTINUE}
\]
\[
\text{GO TO } 4
\]

\[
\text{A(IC+1)}=1.
\]

\[
\text{CALL RROOT FOR DETERMINATION OF THE REAL ROOTS}
\]

\[
\text{CALL RROOT(A,X,N)}
\]

\[
\text{RETURN}
\]

\[
\text{END}
\]

\[
\text{END OF SUBROUTINE GENER}
\]

\[
\text{BEGIN SUBROUTINE GENS}
\]

\[
\text{SUBROUTINE GENS(S,M)}
\]

\[
\text{This subroutine generates the mathematical molecule for the extended Simpsons (1/3) rule or the extended trapezoidal rule. Each element is divided by the total length of the}
\]

---

This text is a description of a subroutine that generates the \( N+1 \) coefficients of the characteristic equation of a tridiagonal matrix. The coefficients are stored in a vector, and an auxiliary vector is used for computation. The subroutine ends with a call to a root-finding subroutine.
C integration interval, thus making it only dependent on the
C number (M-1) of subintervals.
C
DIMENSION S(I)
C
IF (M.LE.1) RETURN
MM=M-1
FAC=MM
IF (M.EQ.(M/2)*2) GO TO 3
C
SIMPSON-S RULE IF M IS ODD
C
  FAC=1./(3.*FAC)
  I=1
  S(I)=FAC
  I=I+1
  S(I)=4.*FAC
  I=I+1
  S(I)=FAC
  IF (I.EQ.M) RETURN
  S(I)=2.*FAC
  GO TO 1

TRAPEZOIDAL RULE IF M IS EVEN
C
3  FAC=1./FAC
   S(I)=FAC/2.
   S(M)=FAC/2.
   IF (MM.LT.2) RETURN
   DO 4 I=2,MM
4   S(I)=FAC
   RETURN
   END

END OF SUBROUTINE GEN

BEGIN SUBROUTINE INIT

SUBROUTINE INIT (XINP, YINP, MINP, XC, YC, YY, ZZ, DMIN)

This subroutine selects starting values for the variables XC/YC
If Xc -as input- is equal to zero. In addition, three vectors
are generated which are needed in subroutine DETFS.

XINP, YINP Coord of the embankment X-section
MINP No. of Coord points XINP/YINP
XC, YC Coord of the center of the arc
DMIN Minimum increment for the variables XC, YC and R
YY Minimum value for YC
ZZ Distance below max YINP down to which YC is permissible
H Thickness of the compressible layer
RHO(I) Slope of the line connecting XINP(I)/YINP(I) and
        XINP(I+1)/YINP(I+1)
TAU TAU=1.+RHO**2
PSI(I) YINP-value of the above line for XINP=0

DIMENSION XINP(1), YINP(1)

COMMON/ SAPOD/ IOUTP, W, H, GLOAD, CLOAD, NARC, NRAD
COMMON/ INDET/ RHO(19), TAU(19), PSI(19)

XX=XINP(MINP)-DMIN
YY=YINP(1)
DO 1 I=2, MINP
J=I-1
RHO(J)=(YINP(I)-YINP(J))/(XINP(I)-XINP(J))
TAU(J)=1.+RHO(J)*RHO(J)
PSI(J)=YINP(J)-RHO(J)*XINP(J)
IF (YINP(J).GT. YY) YY=YINP(I)
1 CONTINUE
YY=YY-ZZ
IF (XC.NE.O.) GO TO 2
YC=YY
R=YY+H
A=YY-YINP(1)
RR=R*R
XC=(XX+SQRT(RR-YY*YY)+SQRT(RR-A*A))/2.
2 RETURN
END

C END OF SUBROUTINE INIT

C BEGIN SUBROUTINE LAGR

SUBROUTINE LAGR (X, Y, M, JST, XX, YY, N)

This subroutine interpolates the values of the function Y(X)
from the known YY(XX) by use of Lagrangian polynomial

X Vector of arguments for which the values of the function are interpolated
Y Vector of interpolated values starting with Y(JST)
M No. of X-s
XX Vector of arguments for which the values of the function are known
YY Vector of known values of the function
N No. of xx-s
RN Auxiliary vector

DIMENSION X(1), Y(1), XX(1), YY(1), RN(101)

JS=JST-1
DO 1 J=1, M
JJ=JS+J
Y(JJ)=0.
1
DO 3 K=1, N
DO 4 J=1, M
4 RN(J)=1.
RD=1.

DO 2 I=1, N
IF (I.EQ.K) GO TO 2
DO 5 J=1, M
5 RN(J)=RN(J)*(X(J)-XX(I))
BEGIN SUBROUTINE MATR

SUBROUTINE MATR (IS, IE, M, XV, A, XM)

Given the vector XV with elements XV(IS), XV(IS+1), ..., XV(IE), this subroutine generates the M by IE-IS+1 matrix XM, whose elements are stored one dimensionally as follows—

XM(1) = 1, XM(2) = (XV(IS) - A), XM(3) = (XV(IS) - A)**2, ..., XM(M) = (XV(IS) - A)**(M-1), XM(M+1) = 1, XM(M+2) = (XV(IS+1) - A), ...

XM(M*(IE-IS+1)) = (XV(IE) - A)**(M-1)

Subtraction of A is done to increase the accuracy.

DIMENSION XV(1), XM(1)

K = 0
DO 1 I = IS, IE
    K = K + 1
    XM(K) = 1
    XVT = XV(I) - A

IF STATEMENT TO INCLUDE CASE M = 1

IF (M .EQ. 1) GO TO 1
DO 2 J = 2, M
    L = K
    K = K + 1
    XM(K) = XM(L) * XVT
1 CONTINUE
RETURN
END

END OF MATR

begin subroutine minv

SUBROUTINE MINV (A, N, D)

This subroutine inverts a general matrix A by means of the standard Gauss-Jordan method.

DIMENSION A(1), L(1600), M(1600)
C
C SEARCH THE LARGEST ELEMENT
C
D=1.0
NK=-N
DO 80 K=1,N
NK=NK+N
L(K)=K
M(K)=K
KK=NK+K
BIGA=A(KK)
DO 20 J=K,N
IZ=N*(J-1)
DO 20 I=K,N
IJ=IZ+I
10 IF (ABS(BICA)-ABS(A(IJ))) 15,20,20
15 BIGA=A(IJ)
L(K)=I
M(K)=J
20 CONTINUE
C
C INTERCHANGE ROWS
C
J=L(K)
IF (J-K) 35,35,25
25 KI=K-N
DO 30 I=1,N
KI=KI+N
HOLD=-A(KI)
JI=KI-K+J
A(KI)=A(JI)
30 A(JI)=HOLD
C
C INTERCHANGE COLUMNS
C
35 I=M(K)
IF (I-K) 45,45,38
38 JP=N*(I-1)
DO 40 J=1,N
JK=NK+J
JI=JP+J
HOLD=-A(JK)
A(JK)=A(JI)
40 A(JI)=HOLD
C
C Divide column by minus pivot (value of pivot element is contained in BIGA)
C
45 IF (BIGA) 48,46,48
46 D=0.0
RETURN
C PAGE P-80
C
48 DO 55 I=1,N
IF (I-K) 50,55,50
50 IK=NK+I
A(IK)=A(IK)/(-BIGA)
55 CONTINUE
C
C REDUCE MATRIX
DO 65 I=1,N  
   IK=NI+I  
   IJ=I-N  
   DO 65 J=1,N  
   IJ=IJ+N  
   IF (I-K) 60,65,60  
   IF (J-K) 62,65,62  
   60 KJ=IJ-I+K  
   A(IJ)=A(IK)*A(KJ)+A(IJ)  
   65 CONTINUE

DIVIDE ROW BY PIVOT

KJ=K-N  
DO 75 J=1,N  
   KJ=KJ+N  
   IF (J-K) 70,75,70  
   70 A(KJ)=A(KJ)/BIGA  
   75 CONTINUE

PRODUCT OF PIVOTS

D=D*BIGA

REPLACE PIVOT BY RECIPROCAL

A(KK)=1.0/BIGA  
80 CONTINUE

FINAL ROW AND COLUMN INTERCHANGE

K=N  
100 K=K-1  
   IF (K) 150,150,105  
105 I=L(K)  
   IF (I-K) 120,120,108  
108 JG=N*(K-1)  
   JR=N*(I-1)  
   DO 110 J=1,N  
   JK=JG+J  
   HOLD=A(JK)  
   JI=JR+J  
   A(JK)=-A(JI)  
   110 A(JI)=HOLD  
120 J=M(K)  
   IF (J-K) 100,100,125  
125 KI=K-N  
   DO 130 I=1,N  
   KI=KI+N  
   HOLD=A(KI)  
   JI=KI-K+J  
   A(KI)=-A(JI)  
130 A(JI)=HOLD  
   GO TO 100  
150 RETURN

END

END OF SUBROUTINE MINV
BEGIN SUBROUTINE MODAL

SUBROUTINE MODAL (LAYER, IBC, N, FIMP, RC, RK, XO, XE, EIG, X, XI, F)

This subroutine generates the coeff. matrix P, determines the
coefficients of the characteristic equation which is then solved to give the eigenvalues EIG, knowing the eigenvalues, the modal matrix X is computed to generate its inverse XI. Generation of P depends on the boundary conditions which are indicated by LAYER, IBC, CHI, RC, RK, XO and XE

C LAYER Index for identification of drainage condition and layer interface if any.
C LAYER=1 Horiz Drainage
C LAYER=2 Vert. drainage, homogeneous soil
C LAYER. GE. 4 Vert. drainage, two layers where layer gives the location of the interface.
C IBC Index for identification of boundary conditions
C IBC=1 Vert. drainage—Impeded drainage at bottom
C IBC=2 Vert. drainage—Free drainage at bottom
C IBC=3 Vert. drainage—No drainage at bottom
C FIMP Impeded drainage factor
C RC Ratio of coeff. of consol. (Lower/upper layer)
C RK Ratio of coeff. of permeability (Lower/upper layer)
C XD Lower boundary of the solution domain. XD=0. if LAYER. GT. 1
CXE Upper boundary of the solution domain. XE=H if LAYER. GT. 1
C EIG Vector of eigenvalues
C X Modal matrix. Matrix of eigenvectors
C XI Inverse of X
C D,F Auxiliary matrices
C P Coeff. matrix
C N No. of eigenvalues=No. of Nodal points in the program numbering system.

DIMENSION P(100), D(50), F(1), EIG(1), X(1), XI(1)

C

IF (IBC. EQ. 4) GO TO 25
IF (LAYER. NE. 2) GO TO 1
IF (IBC. NE. 2) GO TO 2

C COMPUTE EIGEN VALUES AND MODAL MATRIX DIRECTLY FOR LAYER=2 AND IBC=2
C
AN=N+1
AN=3.141592653589793/AN
KJ=0
DO 3 J=1,N
AJ=J
EIG(J)=-2.+2.*COS(AJ*AN)
3 CONTINUE
GO TO 4

C
IF (IBC.NE.3) GO TO 1

COMPUTE EIGEN VALUES AND MODAL MATRIX DIRECTLY FOR LAYER=2 AND IBC=3

AN=2*N
AN=3.141592653589793/AN
KJ=0

DO 5 J=1,N
AJ=2*J-1
EIG(J)=-2.+2.*COS(AJ*AN)

DO 5 K=1,N
KJ=KJ+1
AK=K
X(KJ)=SIN(AK*AJ*AN)
5 CONTINUE
GO TO 4

GENERATE MATRICES P AND D FOR CASES WHERE 2.NE.IBC.NE.3

1)
D(1)=1.0
P(1)=0.0
P(2)=2.0
IF (IBC.LT.4) GO TO 6

CASE OF HORIZONTAL FLOW

25)
AN=2*N
AN=3.141592653589793/AN
KJ=0

DO 7 J=1,N
AJ=2*J-1
EIG(J)=-2.+2.*COS(AJ*AN)

DO 7 K=1,N
KJ=KJ+1
AK=K
X(KJ)=SIN(AK*AJ*AN)
7 CONTINUE
GO TO 4

GENERATE P AND D FOR VERTICAL DRAINAGE

6)
INT=N
IF (LAYER.GT.2) INT=LAYER-2
IF (LAYER.LE.3) I=1
GO TO 27

DO 9 I=2,INT
IE=2*I
P(IE-1)=-1.
P(IE)=2.
D(I)=1.
9 CONTINUE
IF (Int. NE. N) GO TO 10
P(IE)=2.-FIMP
GO TO 8

C
C COEFFICIENTS OF P AND D FOR THE LAYERED CASE
C
10 PO=-RC*RC
PE=2.*RC
FIN=PE/(RC+RK)
P(IE+1)=-FIN
P(IE+2)=FIN*(1.+RK)
D(INT+1)=FIN
P(IE+3)=-FIN*RC*RK
P(IE+4)=PE
D(INT+2)=FIN*RC
INT=INT+3

C
DO 11 I=INT,N
IE=2*I
P(IE-1)=PO
P(IE)=PE
D(I)=RC*D(I-1)
11 CONTINUE
C
IF (IBCV. NE. 3) GO TO 12
P(IE-1)=2.*PO
D(N)=2.*D(N)
GO TO 8
C
12 IF (IBCV. EQ. 1) P(IE)=RC*(2.-FIMP)
C
Call GENER to generate the characteristic equation and to compute
the eigenvalues EIG

C
B CALL GENER (P,F,EIG,N)
C
C Compute eigenvectors from P,D and EIG
C
MEND=N-1
NN=2*N
DO 15 K=1,N
NK=N*K
X(NK)=1.
X(NK-1)=P(NN)+EIG(K)
15 CONTINUE
C
DO 16 ME=2,MEND
NN=NN-2
DO 16 K=1,N
NK=N*K-ME
X(NK)=P(NN+1)*X(NK+2)+(P(NN)+EIG(K))*X(NK+1)
16 CONTINUE
C
C Premultiply matrix X by matrix D
C Restore the X-elements since MINV destroys the original matrix
C
I=0
DO 20 J=1,N
DO 20 K=1,N
I=I+1
X(I)=X(I)*D(K)
4
NN=N*N
DO 17 I=1,NN
XI(I)=X(I)
17 CONTINUE

C CALL MINV FOR INVERSION
CALL MINV (XI, N, DET)
RETURN
END

END OF SUBROUTINE MODAL

SUBROUTINE MPRD BEGINS

This subroutine premultiplies the M*L matrix B by the N*M matrix A and stores the result in the N*L matrix R. The first elements of the matrices are A(IAS), B(IBS), R(IRS). The normal case will be that where IAS=IBS=IRS=1. If all matrices are stored one dimensionally, the following formula for the element R(IR) is obtained— R(IR)=R(J+(K-1)*N+IRS-1)=SUM(I=1,M) of A(J+(I-1)*N+IAS-1)*B(I+(K-1)*M+IBS-1)

DIMENSION A(1), B(1), R(1)

IR=IRS-1
KM=IBS-M-1
DO 1 K=1,L
KM=KM+M
DO 1 J=1,N
IR=IR+1
IA=J+IAS-N-1
IB=KM
R(IR)=0.
DO 1 I=1,M
IA=IA+N
IB=IB+1
R(IR)=R(IR)+A(IA)*B(IB)
1 CONTINUE
RETURN
END

END OF MPRD

BEGIN RROOT

SUBROUTINE RROOT (COF, XR, M)

This subroutine computes the real roots of an M-th degree polynomial. COF is the coeff. vector with M+1 elements. XR is the vector containing the M roots. M must be greater than 2 but less than 50 because of storage allocation. The polynomial has the form F(X)=0. = COF(1)+COF(2)*X+......+COF(M+1)*X**M
A, B, C are auxiliary vectors of length M+1
C EPS gives the required accuracy.

DIMENSION COF(1), XR(1), A(50), B(50), C(50)

EPS=1./1000.
N=M
NN=M+1
X=0.

C Rename original coeff. for final iteration.

DO 1 J=1, NN
A(J)=COF(J)
1 CONTINUE

C Apply Newtons rule X(J+1)=X(J)-F(X(J))/((D?DX)(F(XJ)))
and obtain the values of the function and its derivative
for the guess X(J) from Horner's scheme. The roots are
always approximated from above and the last root is used
as initial guess for the reduced polynomial.

2 B(NN)=A(NN)
C(NN)=A(NN)
I=NN

DO 3 J=2, N
I=I-1
B(I)=A(I)+X*B(I+1)
C(I)=B(I)+X*C(I+1)
3 CONTINUE
B(1)=A(1)+X*B(2)

C Newtons rule and accuracy check

DX=-B(1)/C(2)
X=X+DX
EPAB=EPS*ABS(X)
IF (ABS(DX).GT.EPAB) GO TO 4
XR(N)=X

C Define coeff. of the reduced polynomial.

DO 6 J=1, N
A(J)=B(J+1)
6 CONTINUE
NN=N
N=N-1
IF (N.GT.1) GO TO 2
XR(1)=-A(1)/A(2)

C The roots are now stored as XR(1), LT. XR(2), LT. .. LT. XR(M)
C Make the final iteration using the original polynomial

MM=M-1
DO 7 K=1, MM
I=M+1
DO 8 J=2, M
I=I-1
B(I)=COF(I)+XR(K)*B(I+1)
C(I)=B(I)+XR(K)*C(I+1)
8 CONTINUE
IF (N.GT.1) GO TO 2
XR(1)=-A(1)/A(2)
CONTINUE

\[ B(1) = \text{COF}(1) + X\text{R}(K) \cdot B(2) \]
\[ DX = -B(1)/C(2) \]
\[ X\text{R}(K) = X\text{R}(K) + DX \]
\[ \text{EPABK} = \text{EPS} \cdot \text{ABS}(X\text{R}(K)) \]

IF (ABS(DX) GT EPABK) GO TO 9

CONTINUE

RETURN

END

END OF RROOT

SUBROUTINE STAB

SUBROUTINE STAB (XC, YC, R, XINP, YINP, MINP, MX, MYE, SU, FX, D, DM, YY)

This subroutine searches automatically for the smallest factor of safety starting with the initial data set XC, YC, R.

The parameters have the following significance:

XC, YC  Coord of the center of the arc
R       Radius of the arc
XINP    Coord of the points describing the x-section of the embankment
YINP    the embankment
MINP    No. of XINP/YINP points
MX       No. of equally spaced points in X-dirn
MYE      No. of equally spaced points in Y-dirn
SU       Vector of shear strengths with MX*MYE elements
FX       Minimum factor of safety
D        Maximum step size be used in the search program
DM       Minimum step size be used in the search program
YY       Minimum permissible value for YC
X, Y, Z  Auxiliary vectors

DIMENSION XINP(1), YINP(1), SU(1), X(2), Y(2), Z(2)

Evaluate safety factor at initial base point:

\[ X(1) = YC \]
\[ X(2) = XC \]
\[ KEN = -1 \]

11

KEN = KEN + 1

DEL = D

CALL VARYR (X(1), X(2), R, XINP, YINP, MINP, MX, MYE, SU, FX, DM, YY)

4

FS = FX

DO 1 I = 1, 2

Y(I) = X(I)

1

Z(I) = X(I)

EXPLORATORY MOVES

DO 2 I = 1, 2

Y(I) = Z(I) + DEL

CALL VARYR (Y(I), Y(2), R, XINP, YINP, MINP, MX, MYE, SU, FY, DM, YY)

IF (FY, LT, FS) GO TO 5
Y(I) = Z(I) - DEL
CALL VARYR (Y(1), Y(2), R, XINP, YINP, MINP, MX, MYE, SU, FY, DM, YY)
IF (FY.LT.FS) GO TO 5
Y(I) = Z(I)
GO TO 2
5
FS = FY
2
CONTINUE
IF (FS.LT.FX) GO TO 6
IF (DEL.LE.DM) GO TO 10
DEL = DEL/2.
GO TO 4
C
C PATTERN MOVE
C
6
DO 3 I=1,2
A = Y(I) - X(I)
IF (A) 7, 8, 9
7
A = -2.*DEL
GO TO 8
9
A = 2.*DEL
8
B = X(I)
X(I) = Y(I)
Y(I) = B + A
3
CONTINUE
FX = FS
CALL VARYR (Y(1), Y(2), R, XINP, YINP, MINP, MX, MYE, SU, FS, DM, YY)
IF (FS.LT.FX) GO TO 6
GO TO 4
C
C Start new search if the circle giving the minimum safety factor
C so far does not outcrop at or in front of the embankment toe
C
10
IF (KEN.EQ.1) GO TO 12
FMIN = FX
13
YC = X(1)
XC = X(2)
IF (KEN.EQ.1) GO TO 14
IF (((XC+SQRT(R*R-YC*YC)).LT.XINP(MINP)) .AND. XINP(MINP)) GO TO 11
GO TO 14
12
IF (FMIN.GT.FX) GO TO 13
FX = FMIN
14
RETURN
C
END
C
C END OF STAB
C
C
SUBROUTINE VARYR (YC, XC, R, XINP, YINP, MINP, MX, MYE, SU, FS, DMIN, YY)
C
C This subroutine evaluates the factors of safety for NRAD
C trial arcs with the same center XC/YC, but different radii.
C
DIMENSION XINP(1), YINP(1), SU(1), F(10)
DIMENSION C(6)
COMMON/ SAPDD/ IOUTP, W, H, OLOAD, CLOAD, NARC, NRAD
C
Arcs whose centers lie below YY are not considered
IF (YC.LT.YY) go to 10

Determine minimum and maximum radi possible

RMIN=YC
IF (XC.LT.XINP(MINP)) GO TO 1
AI=XC-XINP(MINP)+DMIN
RMIN=SQRT(AI*AI+YC*YC)
1 RMAX=YC+H
AI=YC-YINP(1)
AI=SQRT(AI*AI+XC*XC)
IF (RMAX.GT.AI) RMAX=AI
IF (RMAX.GE.RMIN) GO TO 2
10 R=0.
FS=.1E36
FS=1.0E35
GO TO 3
2 R=RMAX

Determine the factor of safety for the maximum radius

CALL DETFS (XC,YC,R,XINP,YINP,MINP,MX,MYE,SU,FS)
IF (RMAX.LE.(1.02*RMIN)) GO TO 3
NN=NRAD-1
IF (NN.NE.0) GO TO 3
AI=NN
DELTA=(RMAX-RMIN)/AI

Determine the factors of safety for arcs with radii
RR=RMIN+(I-1)*DELTA, and store them in vector F at place 2*I-1

RR=1.00001*RMIN-DELTA
DO 4 J=1,NN
RR=RR+DELTA
CALL DETFS (XC,YC,RR,XINP,YINP,MINP,MX,MYE,SU,F(J))
4 CONTINUE

Search for the minimum factor of safety which is then returned to the calling program together with the corresponding radius

DO 5 I=1,NN
IF (F(I).GE.FS) GO TO 5
FS=F(I)
AI=I-1
R=RMIN+AI*DELTA
5 CONTINUE
3 RETURN
END

END OF VARYR

BEGIN SUBROUTINE EFGEN

SUBROUTINE EFGEN (PSI,T,EIG,IVAR,MM,NN,D,LI)

This subroutine generates the time-dependent diagonal matrix D
C PSI Vector which considers the influence of the soil parameters
C T Time for which the diagonal matrix D is generated
C EIG Vector of eigen values
C IVAR=0 Const. soil parameters
C IVAR=1 Variable soil parameters
C MM= No. of points XT for which D must be evaluated
C NN No. of eigenvalues
C D Diagonal matrix to be returned

DIMENSION PSI(1),EIG(1),D(1)

C

EF=64.
IF (T. NE. 0.) GO TO 7
LAST=MM*NN
DO 8 I=1,LAST
 8 D(I)=1.0
RETURN
7 IF (IVAR.EQ.0) GO TO 1

C VARIABLE SOIL PARAMETERS

II=0
DO 2 J=1,MM
PSIT=PSI(J)*T
DO 2 I=1,NN
  if (MM.EQ.1) PSIT=PSI(I)*T
  II=II+1
    D(II)=10.**((PSIT*EIG(I)/2.302585)/EF)
 2 CONTINUE
RETURN

C CONSTANT SOIL PARAMETERS

1 PSIT=PSI(1)*T
  DO 3 I=1,NN
    D(I)=10.**((PSIT*EIG(I)/2.302585)/EF)
  IF (MM.LT.2) GOTO 3
1 II=I
  DO 6 J=2,MM
    II=II+NN
6 D(II)=D(I)
3 CONTINUE
RETURN

C END

C END OF SUBROUTINE EFGEN